SUPPLEMENTAL
GEOTECHNICAL EXPLORATION

PACE ROAD INDUSTRIAL FACILITY
POLK CITY, POLK COUNTY, FLORIDA

UES PROJECT NO. A26139.00190.001
UES REPORT NO. 2163464

PREPARED FOR:

Pace Road DC Development, LLC
c/o Hillwood, A Perot Company
5050 West Tilghman Street
Allentown, Pennsylvania 18104

Attention: Mr. Mac Utsey, Vice President

PREPARED BY:

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March 25, 2026

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Pace Road DC Development, LLC
c/o Hillwood, A Perot Company
5050 West Tilghman Street
Allentown, Pennsylvania 18104

Attention: Mr. Mac Utsey, Vice President
Mac.Utsey@hillwood.com

Reference: **Supplemental Geotechnical Exploration**
Pace Road Industrial Facility
Auburndale, Polk County, Florida
UES Project No. A26139.00190.001
UES Report No. 2163464

Dear Mr. Utsey:

UES Professional Solutions, LLC d/b/a UES has completed a supplemental geotechnical exploration at the above referenced site in Polk County, Florida. The scope of our exploration was planned in conjunction with and authorized by you. This exploration was performed in accordance with UES Proposal No. 2162200v3, dated February 13, 2026 and with generally accepted soil and foundation engineering practices. No other warranty, express or implied, is made.

The following report presents the results of our field exploration with a geotechnical engineering interpretation of those results with respect to the project characteristics as provided to us. We have included soil descriptions at boring locations, our estimates of the seasonal high groundwater level at the boring locations, and geotechnical recommendations for foundation design, pavement design, site preparation, and stormwater system design. *The site was found to be generally suitable for the proposed development following typical site preparation procedures.*

We appreciate the opportunity to have worked with you on this project and look forward to a continued association. Please do not hesitate to contact us if you should have any questions, or if we may further assist you as your plans proceed.

Respectfully Submitted,
UES PROFESSIONAL SOLUTIONS, LLC
Florida Engineering Business Registry No. 549

Ricardo Kiriakidis, Ph.D., P.E.
Geotechnical Department Manager

ZACHARY W. ADAMS LICENSE NO. 82935 STATE OF FLORIDA PROFESSIONAL ENGINEER

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1.0 PROJECT DESCRIPTION

UES understands that Pace Road DC Development, LLC is in the design process for Phase 2 of the warehouse development located at 810 Pace Road in Polk City, Polk County, Florida. We were provided with a conceptual site plan showing the proposed site improvements. The site plan identified two (2) warehouse structures totaling approximately 462,000 square feet, four (4) dry bottom stormwater ponds, three (3) substation areas, one (1) 4 bay HV switching area, and associated paved parking and drive areas.

UES previously completed a preliminary geotechnical exploration for the entire site and a supplemental exploration for Phase 1. Our findings were presented in UES Report Nos. 1957237 and 2086203, dated June 3, 2022, and April 30,2024, respectively.

At this time, UES has been requested to perform a supplemental geotechnical exploration of the subject site for design purposes. Our scopes of services have been designed to evaluate the subsurface conditions on subject site for the proposed site improvements. We have used our previously obtained data in conjunction with our current exploration to provide geotechnical recommendations for Phase 2 of the development.

We understand maximum column loads will be on the order of 50 kips for the proposed equipment pad and service yard structures and on the order of 550 kips for the data center warehouse buildings. Maximum wall loads will be on the order of 5 kips per linear foot for the proposed service yard structures. We have assumed that cuts/fills on the order of 3 feet or less will be necessary to achieve finished grades in the proposed building and pavement areas of the site.

Should any of the above information or assumptions made by UES be inconsistent with the planned development and construction, we request that you contact us immediately to allow us the opportunity to review the new information in conjunction with our report and revise or modify our engineering recommendations accordingly, as needed.

No site or project facilities/improvements, other than those described herein, should be designed using the soil information presented in this report. Moreover, UES will not be responsible for the performance of any site improvement so designed and constructed.

2.0 PURPOSE

The purposes of this exploration were:

- to explore and evaluate the subsurface conditions at the site with special attention to potential problems that may impact the proposed development,
- to provide our estimates of the seasonal high groundwater level at the boring locations,
- to provide geotechnical engineering recommendations for foundation design, pavement design, site preparation, and stormwater design.

This report presents an evaluation of site conditions on the basis of geotechnical procedures for site characterization. The recovered samples were not examined, either visually or analytically, for chemical composition or environmental hazards. We would be glad to provide you with a proposal for these services at your request.

Our exploration was not designed to specifically address the potential for surface expression of deep geological conditions, such as sinkhole development related to karst activity. This evaluation requires a more extensive range of field services than those performed in this study. We would be pleased to conduct an exploration to evaluate the probable effect of the regional geology upon the proposed construction, if you so desire.

3.0 SITE DESCRIPTION

The subject site is located within Section 17, Township 27 South, and Range 25 East in Polk County, Florida. More specifically, the subject site is located at 910 Pace Road in Polk City, Florida, as shown in the attached Figure A-1. At the time of drilling, the site consisted of a grass covered, vacant parcel.

3.1 SOIL SURVEY

There are four (4) native soil types mapped within the general vicinity of the site according to the USDA NRCS Soil Survey of Polk County. A brief summary of the mapped surficial soil type(s) is presented in Table I below. *Please note that the native soil types and their associated engineering properties have likely been altered by past development in the vicinity of the site.*

TABLE I
SUMMARY OF PUBLISHED SOIL DATA [1]

Soil Symbol	Soil Type	Hydrologic Group	Drainage Characteristics	Depth of Published Seasonal High GWT (feet)
2	Apopka fine sand, 0 to 5 percent slopes	A	Well drained	>6½
3	Candler sand, 0 to 5 percent slopes	A	Excessively drained	>6½
14	Sparr sand, 0 to 5 percent slopes	A/D	Somewhat poorly drained	1½ to 3½
15	Tavares fine sand, 0 to 5 percent slopes	A	Moderately well drained	3½ to 5

[1] Data obtained from the NRCS online webpage, accessed on 3/23/2026

3.2 TOPOGRAPHY

According to information obtained from the United States Geologic Survey (USGS) "Polk City, Florida" quadrangle map, the predevelopment ground surface elevation across the site area ranges from approximately +170 to +175 feet National Geodetic Vertical Datum (NGVD). A copy of the portion of the USGS Map is included in Appendix A.

4.0 SCOPE OF SERVICES

The services conducted by UES during our geotechnical explorations were as follows:

- Drilled thirty-nine (39) Standard Penetration Test (SPT) borings within the proposed building, pavement, substation, HV switching area, and stormwater pond footprints of Phase 1 & 2 to depths of 10 to 50 feet below existing grades in March 2026.

- Drilled thirty-five (35) Standard Penetration Test (SPT) borings within the proposed building, pavement, lift station, and stormwater pond footprints of Phase 1 to depths of 10 to 30 feet below existing grades in April 2024.

- Drilled sixteen (16) Standard Penetration Test (SPT) borings scattered across Phases 1 & 2 to depths of 25 feet below existing grades in May 2022.

- Secured samples of representative soils encountered in the soil borings for review, laboratory analysis and classification by a Geotechnical Engineer.

- Measured the existing site groundwater levels and provided an estimate of the seasonal high groundwater level at the boring locations.

- Conducted laboratory testing on selected soil samples obtained in the field to determine their engineering properties.

- Assessed the existing soil conditions with respect to the proposed construction.

- Prepared a report which documents the results of our exploration and analysis with preliminary geotechnical engineering recommendations.

5.0 FIELD EXPLORATION

The SPT soil borings were performed with an ATV-mounted drilling rig. Horizontal and vertical survey control was not provided for the test locations prior to our field exploration program. UES located the test borings by using the provided site plan, measuring from existing on-site landmarks shown on an aerial photograph, and by using handheld GPS devices. The indicated test locations should be considered accurate to the degree of the methodologies used. The approximate boring locations are shown in Appendix B.

During our current exploration program, we located and performed thirty-nine (39) SPT borings, designated B-28 through B-55, P-17 through P-20, and SW-08 through SW-14, as shown on the attached Boring Location Plan in Appendix B. The SPT borings were performed in general accordance with the procedures of ASTM D 1586 "Standard Method for Penetration Test and Split-Barrel Sampling of Soils". SPT sampling was performed continuously within the top 10 feet to detect variations in the near surface soil profile.

Ground surface elevations at the boring locations would be beneficial to help us to identify any anomalies in our measured and estimated seasonal high groundwater levels, as well as improve the usefulness of the groundwater information during the civil engineering design of the site.

6.0 LABORATORY TESTING

The soil samples recovered from the test borings were returned to our laboratory and visually classified in general accordance with ASTM D 2487 "Standard Classification of Soils for Engineering Purposes" (Unified Soil Classification System). We selected representative soil samples from the borings for laboratory testing to aid in classifying the soils and to help to evaluate the general engineering characteristics of the site soils. The results of these tests are shown on the boring logs in Appendix B. A summary of the tests performed is shown in Table II on the following page.

TABLE II
LABORATORY METHODOLOGIES

Test Performed	Number Performed	Reference
Grain Size Analysis (#200 wash only)	24	ASTM D 1140 "Amount of Material in Soils Finer than the No. 200 (75 - µm) sieve"
Permeability Test	7	Using the D10 method obtained from a Full Gradation
Moisture Content	31	ASTM D 2216 "Laboratory Determination of Water (Moisture) Content of Soil by Mass"

7.0 SUBSURFACE CONDITIONS

7.1 GENERALIZED SOIL PROFILE OF SOIL BORINGS

The results of our field exploration and laboratory analysis, together with pertinent information obtained from the SPT borings, such as soil profiles, penetration resistance and groundwater levels are shown on the boring logs included in Appendix B. The Key to Boring Logs, Soil Classification Chart is also included in Appendix B. The soil profiles were prepared from field logs after the recovered soil samples were examined by a Geotechnical Engineer. The stratification lines shown on the boring logs represent the approximate boundaries between soil types, and may not depict exact subsurface soil conditions. The actual soil boundaries may be more transitional than depicted. A generalized profile of the soils encountered at our boring locations during our current exploration program is presented in Table III below. For detailed soil profile, please refer to the attached boring logs.

TABLE III
GENERALIZED SOIL PROFILE

Stratum No.	Typical Depth (feet, bls)		Soil Description	Typical Range of SPT "N" Values
	From	To		
1	Surface	4 to 18½	Very loose to dense fine SAND [SP] and fine SAND with silt and clay [SP-SM, SP-SC]	3 to 31
2	4 to 18½	50*	Very loose to very dense silty/clayey fine SAND [SM, SC, SC-SM] with occasional interbedded layer of fine SAND with silt & clay [SP-SM, SP-SC]	2 to 50+

*Denotes maximum termination depth of the borings

8.0 GROUNDWATER CONDITIONS

8.1 EXISTING GROUNDWATER LEVEL

We measured the water levels in the boreholes on March 6, 2026, through March 18, 2026, during our current exploration program. Groundwater was encountered at depths ranging from approximately 12½ to greater than 15 feet below existing grades at our boring locations at the time of our field explorations. The groundwater level encountered at each of the boring locations is shown on the attached boring logs in Appendix B.

Fluctuations in groundwater levels should be anticipated throughout the year, primarily due to seasonal variations in rainfall, surface runoff, and other factors that may vary from the time the borings were conducted.

8.2 SEASONAL HIGH GROUNDWATER LEVEL

Based on historical data, the rainy season in Central Florida is between June and October of the year. In order to estimate the seasonal high water level at the boring locations, many factors are examined, including the following:

- Measured groundwater level
- Drainage characteristics of existing soil types
- Current & historical rainfall data
- Natural relief points (such as lakes, rivers, wetlands, etc.)
- Man-made drainage systems (ditches, canals, retention basins, etc.)
- On-site types of vegetation
- Review of available data (soil surveys, USGS maps, etc.)
- Redoximorphic features (mottling, stripping, etc.)

Based on the results of our field exploration and the factors listed above, we believe that the seasonal high groundwater level will form as a transient, perched condition on top of the hydraulically restrictive silty/clayey soils encountered at varying depths across the site. We estimate that the perched seasonal high groundwater level at our boring locations will generally form at depths ranging from approximately 3½ to greater than 10 feet below existing grades. The estimated seasonal high groundwater levels at the boring locations are shown on the attached boring logs.

Please note, ground surface elevations at the boring locations would be beneficial to allow us to identify any anomalies in both our measured and estimated seasonal high groundwater levels, as well as improve the usefulness the groundwater information during the civil engineering design of the site.

It should be noted that the estimated seasonal high water levels provided should be considered accurate to about ½ foot +/- and do not provide any assurance that groundwater levels will not exceed these estimated levels during any given year in the future. Should the impediments to surface water drainage be present, or should rainfall intensity and duration, or total rainfall quantities, exceed the normally anticipated rainfall quantities, groundwater levels might exceed our seasonal high estimates. Further, it should be understood that changes in the surface hydrology and subsurface drainage from on-site and/or off-site improvements could have significant effects on the normal and seasonal high groundwater levels.

9.0 FOUNDATION DESIGN RECOMMENDATIONS

The following recommendations are made based upon a review of the attached soil test data, our understanding of the proposed construction, and experience with similar projects and subsurface conditions. The applicability of geotechnical recommendations is very dependent upon project characteristics such as improvement locations, and grade alterations. UES must review the final site and grading plans to validate all recommendations rendered herein.

Additionally, if subsurface conditions are encountered during construction, which were not encountered in the borings, report those conditions immediately to us for observation and recommendations.

9.1 Structural and Grading Information

UES understands that the proposed project will consist of the design and construction of a new warehouse facility located in Polk County, Florida. We were provided with a conceptual site plan showing the proposed site improvements. two (2) warehouse structures totaling approximately 462,000 square feet, four (4) dry bottom stormwater ponds, three (3) substation areas, one (1) 4 bay HV switching area, and associated paved parking and drive areas.

We understand maximum column loads will be on the order of 50 kips for the proposed equipment pad and service yard structures and on the order of 550 kips for the data center warehouse buildings. Maximum wall loads will be on the order of 5 kips per linear foot for the proposed service yard structures. We have assumed that cuts/fills on the order of 3 feet or less will be necessary to achieve finished grades in the proposed building and pavement areas of the site.

Prior to finalizing any design, the structural/grading information outlined above should be confirmed by the project structural/civil engineer. This is crucial to our evaluation and estimates of settlements. If any of this information is incorrect or if you anticipate any changes, please inform UES immediately so that we may review and modify our recommendations as appropriate.

9.2 Analysis

Based on the results of the soil borings, the near surface soils within the proposed building area appear to be mostly very loose to dense fine SAND [SP, SP-SC, SP-SM] overlying very loose to very dense silty/clayey fine SAND [SC, SM, SC-SM] to depths of 50 feet bls. It is our opinion that proposed structures can be supported on properly designed and constructed shallow foundation systems. Provided that the site preparation recommendations outlined in this report are followed, the parameters outlined below may be used for foundation design.

Although not required to limit settlements to tolerable levels, vibro-replacement techniques could also be considered for the warehouse structures as an alternative to conventional compaction for cost comparison purposes. Vibro-replacement stone columns would result in much higher allowable foundation bearing pressures which would decrease footing sizes resulting in material savings (i.e. concrete and steel reinforcement) to offset some of the cost of the ground improvement program. Foundations prepared using vibro-replacement (stone column) techniques can typically be designed for an allowable net soil bearing pressure on the order of 6,000 to 8,000 psf +/- pounds per square foot (psf). **The specific depths and groupings of the stone columns should be developed by a geotechnical specialty contractor.**

9.3 Bearing Pressure

Due to the large variation in loading conditions between the data center warehouse buildings and the equipment pads/service yard structures, we have separated the design bearing pressure recommendations for each of the structures. The allowable net soil bearing pressure is that pressure that may be transmitted to the soil in excess of the minimum surrounding overburden pressure.

The allowable bearing pressure should include dead load plus sustained live load. The foundations should be designed for the most unfavorable effects due to the combinations of loads specified in Florida Building Code.

Data Center Warehouse Structures

Provided our suggested site preparation procedures are followed, we recommend designing shallow footing foundations for a **maximum allowable net soil bearing pressure of 3,000 pounds per square foot (psf)**. The allowable net bearing pressure is that pressure that may be transmitted to the soil in excess of the minimum surrounding overburden pressure.

Equipment Pads & Service Yard Structures

Provided our suggested site preparation procedures are followed, we recommend designing shallow footing foundations for a **maximum allowable net soil bearing pressure of 3,500 pounds per square foot (psf)**. The allowable net bearing pressure is that pressure that may be transmitted to the soil in excess of the minimum surrounding overburden pressure.

9.4 Foundation Size

The minimum recommended width for an isolated column footing is 24 inches. For continuous wall or slab on grade foundations, the minimum footing width should comply with the current FLBC, but under no circumstances should it be less than 12 inches. Even though the maximum allowable soil bearing pressure may not be achieved, these width recommendations should control the size of the foundations.

9.5 Bearing Depth

The base of all footings should be at least **24 inches** below finished grade elevation for the data center structures and at least 12 inches below finished grade elevation for the equipment pads and service yard structures in accordance with the FLBC. We recommend stormwater and surface water be diverted from the building exterior, both during and after construction, to reduce the possibility of erosion beneath the exterior footings.

9.6 Bearing Material

The bearing level soils should exhibit a density of at least 95 percent of the maximum dry density as determined by ASTM D 1557 (Modified Proctor) to a depth of at least **2 feet below foundation level** as described in this report. In addition to compaction, the bearing soils must exhibit stability and be free of "pumping" conditions.

9.7 Settlement Estimates

Post-construction settlement of the structure will be influenced by several interrelated factors, such as (1) subsurface stratification and strength/compressibility characteristics of the bearing soils to a depth of approximately twice the width of the footing; (2) footing size, bearing level, applied loads, and resulting bearing pressures beneath the foundation; (3) site preparation and earthwork construction techniques used by the contractor, and (4) external factors, including but not limited to vibration from offsite sources and groundwater fluctuations beyond those normally anticipated for the naturally-occurring site and soil conditions which are present.

Our settlement estimates for the structure are based upon adherence to our recommended site preparation procedures presented in this report. Any deviation from these recommendations could result in an increase in the estimated post-construction settlement of the structures. Furthermore, should building loads change from those assumed by us, greater settlements may be expected.

Due to the sandy nature of the surficial soils, following the compaction operations we expect the majority of settlement to be elastic in nature and occur relatively quickly, on application of the loads, during and immediately following construction. Using the recommended maximum allowable bearing pressure, the assumed maximum structural loads, and the field and laboratory test data which we have correlated into the strength and compressibility characteristics of the subsurface soils, **we estimate the total post-construction vertical settlement of the proposed structure to be on the order of 1 inch or less.**

Differential settlement results from differences in applied bearing pressures and the variations in the compressibility characteristics of the subsurface soils. Assuming our site preparation recommendations are followed, **we anticipate post-construction differential settlement of less than ½ inch.**

9.8 FLOOR SLABS

Conventional floor slabs may be supported upon the compacted fill and should be structurally isolated from other foundation elements or adequately reinforced to prevent distress due to differential movements. For the slab design, we recommend using a subgrade modulus (k) of 100 pounds per cubic inch, which can be achieved by compacting the subgrade soils as recommended in this report. We recommend using a sheet vapor barrier (in accordance with Florida Building Code requirements) beneath the building slab-on-grade to help control moisture migration through the slab.

10.0 PAVEMENT RECOMMENDATIONS

10.1 GENERAL

We assume that the proposed parking and drive areas will consist of a combination of flexible, asphaltic and rigid concrete pavement sections with typical light and some heavy-duty traffic. Our recommendations for both pavement types are listed in the following sections. The following recommendations are based on pavement areas being prepared as recommended in this report.

10.2 ASPHALTIC PAVEMENTS

10.2.1 Layer Components

At the time of this exploration, specific traffic loading information was not provided to us. We have assumed the following conditions for our recommended minimum pavement design.

- the subgrade soils are prepared as described in the Site Preparation section of this report
- a twenty (20) year design life
- terminal serviceability index (P_t) of 2.5
- reliability of 90 percent
- total equivalent 18-kip single axle loads (E_{18}SAL) up to 35,000 for light duty pavements - car and pickup truck traffic
- total equivalent 18-kip single axle loads (E_{18}SAL) up to 250,000 for heavy duty pavements – occasional heavy truck traffic (delivery, trash collection, service lanes, etc.)

We recommend using a three-layer pavement section for the proposed asphaltic parking/drive areas consisting of stabilized subgrade, base course, and surface course. Based on the results of our soil borings, the assumed traffic loading information and review of the 2024 FDOT Flexible Pavement Design Manual, our minimum recommended pavement component thicknesses are presented in Table IV below. Where applicable, the local municipality minimum standards should be followed when more stringent than the recommendations herein.

TABLE IV
MINIMUM ASPHALTIC PAVEMENT COMPONENT THICKNESSES

Service Level	Layer Component		
	Surface Course (inches)	Base Course (inches)	Stabilized Subgrade (inches)
Light Duty	1½	6	12
Heavy Duty	2½	8	12

10.2.2 Subgrade

We recommend that the stabilized subgrade materials immediately beneath the base course exhibit a minimum Limerock Bearing Ratio (LBR) of 40 as specified by Florida Department of Transportation (FDOT). The stabilized subgrade should be compacted to at least 98 percent of the Modified Proctor maximum dry density (ASTM D 1557) value.

Stabilized subgrade can be imported materials or a blend of on-site and imported materials. If a blend is proposed, we recommend that the contractor perform a mix design to find the optimum mix proportions.

Compaction testing of the subgrade should be performed to full depth at a frequency of at least one (1) test per 10,000 square feet.

10.2.3 Base Course

Based on the results of our exploration and our experience in the project area, limerock, soil-cement and recycled crushed concrete are suitable base course materials for this project. However, local municipality standards may govern the use of recycled crushed concrete use as an alternative base course material. We recommend the civil engineer consult with the local municipalities prior to selecting the base course material for this project.

For a limerock base, the base course should be compacted to a minimum density of 98 percent of the Modified Proctor maximum dry density and exhibit a minimum LBR of 100. The limerock material should comply with the latest edition of the Florida Department of Transportation (FDOT) Road and Bridge Construction specifications.

For a soil-cement base, we recommend the contractor perform a soil-cement design with a minimum seven (7)-day strength of 300 pounds per square inch (psi) on the materials he intends to use. Place soil-cement in maximum 6-inch lifts uniform and compact in place to a minimum density of 95 percent of the maximum dry density according to specifications in ASTM D-558, "Moisture Density Relations of Soil Cement Mixtures".

Place and finish the soil-cement according to Portland Cement Association requirements. Final review of the soil-cement base course should include manual "chaining" and/or "soundings" seven days after placement. Shrinkage cracks will form in the soil-cement mixture and you should expect reflection cracking on the surface course.

Recycled Concrete Aggregate (RCA) may provide a cost-effective alternative material in lieu of limerock or soil cement base courses. Local availability, along with municipality standards, typically governs the use of crushed concrete use as an alternative base course material. The advantages of using crushed concrete as a pavement base course include its high strength (stronger than limerock), resistance to groundwater related distress, and lack of reflection cracking caused by thermal expansion and contraction.

If RCA base is used, the base course material should be sourced from an FDOT approved supplier. The base should be compacted to a minimum density of 98 percent of the Modified Proctor maximum dry density and exhibit a minimum LBR of 150. The base material should comply with the latest edition of the FDOT Road and Bridge Construction Specifications.

Compaction testing of the base course should be performed to full depth at a frequency of at least one (1) test per 10,000 square feet,

10.2.4 Surface Course

For the pavements, we recommend that the surfacing consist of FDOT SuperPave (SP) asphaltic concrete. The surface course should consist of FDOT SP-9.5 fine mix for light-duty areas and FDOT SP-12.5 topped with SP-9.5 fine mix for heavy duty areas. The asphalt concrete should be placed within the allowable lift thicknesses for fine Type SP mixes per the latest edition of FDOT, Standard Specifications for Road and Bridge Construction.

The asphaltic concrete should be compacted to an average field density of 93 percent of the laboratory maximum density determined from specific gravity (G_{mm}) methods, with an individual test tolerance of **+2 percent and -1.2% of the design G_{mm}**. Specific requirements for the SuperPave asphaltic concrete structural course are outlined in the latest edition of FDOT, Standard Specifications for Road and Bridge Construction.

Note: If the Designer (or Contract Documents) limits compaction to the static mode only or lifts are placed one-inch thick, then the average field density should be 92 percent, with an individual test tolerance of + 3 percent, and -1.2% of the design G_{mm}.

After placement and field compaction, the wearing surface should be cored to evaluate material thickness and density. Cores should be obtained at frequencies of at least one (1) core per 10,000 square feet of placed pavement, or a minimum of two (2) cores per day's production.

10.2.5 Effects of Groundwater

One of the most critical influences on the pavement performance in Central Florida is the relationship between the pavement base course and the seasonal high groundwater level. Sufficient separation will need to be maintained between the bottom of base course and the anticipated seasonal high groundwater level. We recommend that the seasonal high groundwater and the bottom of the base course be separated by at least 12 inches for RCA and soil-cement base courses, and at least 18 inches for a limerock base course.

The separation should be confirmed by reviewing the final site grading and paving plan. If the separation is not provided by grading, the installation of underdrains will be required.

10.2.6 Landscape Underdrains

In the event that landscape areas adjacent to the pavements include large mounds (>1 foot) of poorly draining organic topsoils or silty/clayey sands, we recommend that landscape drains be provided to protect the roadway against adverse effects from over-irrigation or excess rainfall. Poorly draining silty and clayey material causes the irrigation and rainwater to perch and migrate laterally into the pavement components, which eventually compromises the integrity of the pavement section.

10.3 Concrete "Rigid" Pavements

Concrete pavement is a rigid pavement that transfers much lighter wheel loads to the subgrade soils than a flexible asphalt pavement; therefore, requiring less subgrade preparation. Concrete pavement is recommended in truck court areas, under the dumpster areas, and 10 feet in front of the trash enclosures, at a minimum.

We recommend using the existing surficial sands or approved structural fill densified to at least 98 percent of Modified Proctor test maximum dry density (ASTM D 1557) without additional stabilization under concrete pavement, with the following stipulations:

1. Prior to placement of concrete, the subgrade soils should be prepared as recommended in The Site Preparation section of this report

2. The surface of the subgrade soils must be smooth, and any disturbances or wheel rutting corrected prior to placement of concrete.

3. The subgrade soils must be moistened prior to placement of concrete.

4. Concrete pavement thickness should be uniform throughout, with exception to the thickened edges (curb or footing).

5. The bottom of the pavement should be separated from the seasonal high groundwater level by at least 12 inches.

Based on the results of our exploration and review of the FDOT Rigid Pavement Design Manual, our recommended minimum concrete pavement design is shown in Table V below.

TABLE V
MINIMUM CONCRETE PAVEMENT THICKNESSES

Service Level	Minimum Pavement Thickness	Maximum Control Joint Spacing	Recommended Saw Cut Depth
Light Duty	6 inches	12 feet x 12 feet	2 inches
Heavy Duty	7 inches	14 feet x 14 feet	2⅓ inches

We recommend using concrete with a minimum 28-day compressive strength of at least 4,000 pounds per square inch. Layout of the Saw cut control joints should form square panels, and the depth of saw cut joints should be ⅓ of the concrete slab thickness.

We recommend allowing UES to review and comment on the final concrete pavement design, including section and joint details (type of joints, joint spacing, etc.), prior to the start of construction.

For further details on concrete pavement construction, please reference the "Guide to Jointing of Non-Reinforced Concrete Pavements" published by the Florida Concrete and Products Association, Inc., and "Building Quality Concrete Parking Areas", published by the Portland Cement Association.

Specimens to verify the compressive strength of the pavement concrete should be obtained for at least every 50 cubic yards, or at least once for each day's placement, whichever is greater.

11.0 SITE PREPARATION

We recommend normal, good practice site preparation procedures for the new construction areas. These procedures include stripping/clearing of the site to remove existing vegetation, roots, topsoils, organics, debris, utility lines, etc. Following stripping, the exposed subgrade soils should be proof-rolled, and all subgrade and subsequent fill/backfill soils should be properly densified.

A more detailed description of this work is presented in this section.

1. Prior to construction, existing underground utility lines within the construction areas should be located. It should be noted that if underground pipes are not properly removed or plugged, they may serve as conduits for subsurface erosion which may lead to excessive settlement of overlying structures.

2. If necessary, perform any remedial dewatering prior to any earthwork operations. Dewatering should be performed to a depth of at least **2 feet below the bottom of any excavations or compacted surface**.

3. Strip the proposed construction limits of existing vegetation, topsoil, roots, organic soils, debris and other deleterious materials within and 5 feet beyond the perimeter of the new construction areas. Expect clearing and grubbing to depths of 6 to 12 inches. We strongly recommend that the stripped/excavated surfaces be observed and probed by representatives of UES.

4. Proof-roll the exposed subsurface soils under the observation of UES, to locate any soft areas of unsuitable soils, and to increase the density of the shallow loose fine sand soils. If deemed necessary by UES, in areas that continue to "yield", remove any deleterious materials and replace with a clean, compacted sand backfill.

5. After approval of the stripped surface, within the building areas, compact the upper **2 feet of the exposed subgrade soils** (including the 5 feet margin) to at least 95 percent of the Modified Proctor test maximum dry density (ASTM D 1557).

6. Place fill/backfill as necessary. All fill should consist of clean sand with less than 10 percent soil fines and be free of organics, debris and other deleterious materials. Fill soils containing between 5 and 10 percent fines may require strict moisture control. Place fill in maximum 12-inch loose, uniform lifts and compact each lift at least 95 percent of the Modified Proctor maximum dry density.

7. Within the pavement areas, the upper 12 inches of subgrade beneath the base course or concrete slabs (sub-base) should be stabilized and compacted to at least 98 percent of the Modified Proctor maximum dry density.

8. Test the subgrade and each lift of fill for compaction at a frequency of not less than one test per 2,500 square feet in the building areas and 10,000 square feet of pavement areas, with a minimum of 4 tests in each area.

9. Prior to the placement of reinforcing steel and concrete, verify compaction within the footing trenches to a depth of 2 feet. We recommend testing every column footing and at least one test every 100 feet of wall footing, with a minimum of 4 tests per building. Re-compaction of the foundation excavation bearing level soils, if loosened by the excavation process, can typically be achieved by making several passes with a walk-behind vibratory sled or jumping jack.

Stability of the compacted soils is essential and independent of compaction and density control. If the near surface soils or the structural fill experience "pumping" conditions, terminate all earthwork activities in that area. Pumping conditions occur when there is too much water present in the soil-water matrix. Earthwork activities are actually attempting to compact the water and not the soil. The disturbed soils should be dried in place by scarification and aeration prior to any additional earthwork activities.

Vibrations produced during vibratory compaction operations at the site may be significantly noticeable within 100 feet and may cause distress to adjacent structures if not properly regulated. Provisions should be made to monitor these vibrations so that any necessary modifications in the compaction operations can be made in the field before potential damages occur. UES can provide vibration monitoring services to help document and evaluate the effects of the surface compaction operation on existing structures. It is recommended that large vibratory rollers remain a minimum of 50 feet from existing structures. Within this zone, the use of a static roller or small hand guided plate compactors is recommended.

12.0 STORMWATER MANAGEMENT DESIGN

We understand that this project will include the construction of four (4) stormwater ponds and two (2) potential exfiltration systems located within the southern portion of the site. Eleven (11) soil borings, designated SW-01 through SW-04 and SW-08 through SW-14, were performed within the proposed stormwater pond footprints during our preliminary & supplemental explorations. Our recommended design parameters are shown in Tables VI and VII on the following page.

TABLE VI
STORMWATER SYSTEM DESIGN PARAMETERS

Design Parameter	Estimated Values					
Relevant Boring Logs	SW-01	SW-02	SW-03	SW-04	SW-08	SW-09
Estimated Depth to Base of Surficial Aquifer (feet)	13½*	13½*	13½*	15**	13½*	5½*
Estimated Fillable Porosity of Surficial in-situ sands (percent)	20	20	20	20	20	20
Estimated Seasonal High Groundwater Level (feet)	13	13	13	10+	12	5
Estimated Avg. Horizontal Saturated Hydraulic Conductivity of Surficial Aquifer (feet per day)	16	22	19	17	38	24
Estimated Vertical Unsaturated Hydraulic Conductivity of Surficial Aquifer (feet per day)	11	15	13	11	25	16

*Depth to base of surficial aquifer based on depth to hydraulically restrictive silty/clayey soil [SC, SM]
**Depth to base of surficial aquifer based on termination depth of boring

TABLE VII
STORMWATER SYSTEM DESIGN PARAMETERS

Design Parameter	Estimated Values				
Relevant Boring Logs	SW-10	SW-11	SW-12	SW-13	SW-14
Estimated Depth to Base of Surficial Aquifer (feet)	18½*	7*	7*	7*	13½*
Estimated Fillable Porosity of Surficial in-situ sands (percent)	20	20	20	20	20
Estimated Seasonal High Groundwater Level (feet)	12½	6½	6½	6½	13
Estimated Avg. Horizontal Saturated Hydraulic Conductivity of Surficial Aquifer (feet per day)	20	24	25	9	21
Estimated Vertical Unsaturated Hydraulic Conductivity of Surficial Aquifer (feet per day)	14	16	17	6	14

*Depth to base of surficial aquifer based on depth to hydraulically restrictive silty/clayey soil [SC, SM]
**Depth to base of surficial aquifer based on termination depth of boring

Please note that the depths presented in Tables VI and VII are referenced to the existing ground surface at the time of our exploration. The hydraulic conductivity values were based on results of laboratory testing. Appropriate factors of safety should be included in the design. UES can perform the drawdown/recovery analyses once the final system configurations and treatment volumes are established, if desired.

13.0 PRELIMINARY SINKHOLE ASSESSMENT

As part of our scope of work, UES was asked to review readily available published literature sources and provide a professional opinion regarding the potential for future sinkhole formation at the subject site. It is important to understand that from a regional context, sinkholes are a characteristic feature of Central Florida geology and can occur anywhere in the region depending on the presence of cavernous voids in the limestone bedrock. We have reviewed available data from the United States Geological Survey (USGS) and Florida Geological Survey (FGS).

The FGS maintains an inventory of reported sinkholes as a continuation of the work originally performed by the Florida Sinkhole Research Institute. This sinkhole database is generally considered to be a record of more recent sinkholes and does not incorporate older and more mature karst features such as large lakes. The FGS database is not a definitive or authoritative resource and should only be used for a generalized overview of recent sinkhole relative density.

Our assessment included a search of the FGS database for sinkholes within a 5-mile radius of the subject site. The closest sinkhole was reported to the north of the subject property at a distance of approximately 2.0 miles. There were nine (9) additional sinkholes reported within 5 miles of the site. We note that the FGS database only contains more recently reported and recorded sinkhole events. A USGS Quadrangle Map presenting the FGS recorded subsidence locations local to the subject property is presented in Appendix A-2.

It is important to understand that sinkholes are a geologic hazard in areas of Florida similar to seismic hazards in areas of California. The future occurrence of sinkholes is very difficult at best, if not impossible to predict even with extensive geological research and site testing. Therefore, one of the best predictors for future sinkhole activity is past sinkhole activity in the vicinity of a site. Based upon the results of our literature review and our previous experience in the general vicinity of the subject site, it is our opinion that based upon a relative scale of low, moderate, high, and very high, the subject site is located within an area with a relatively ***"low"*** propensity for sinkhole formation. We believe that overall, the sinkhole potential on this site is no greater than that of the adjacent existing developments and undeveloped land due to similar underlying geology.

14.0 DEWATERING AND EXCAVATION CONSIDERATIONS

Depending upon the time of year construction commences and the depth of excavation required, some dewatering **may be** required for the successful construction of this project. Where excavations will extend only a few feet below the groundwater table, a sump pump may be sufficient to control the groundwater table. Deeper excavations may require well points and/or sock drains to control the groundwater table. Regardless of the method(s) used, we recommend drawing down the water level at least 2 feet below the bottom of the excavation. The actual method(s) of dewatering should be determined by the contractor. The design and discharge of the dewatering system must be performed in accordance with applicable regulatory criteria (i.e. water management district, etc.) and compliance with such criteria is the sole responsibility of the contractor.

Excavations should be sloped as necessary to prevent slope failure and to allow backfilling. As a minimum, temporary excavations below 4-foot depth should be sloped in accordance with OSHA regulations. Where lateral confinement will not permit slopes to be laid back, the excavation should be shored in accordance with OSHA requirements. During excavation, excavated material should not be stockpiled at the top of the slope within a horizontal distance equal to the excavation depth. Provisions for maintaining workman safety within excavations is the sole responsibility of the contractor.

15.0 CONSTRUCTION RELATED SERVICES

We recommend the owner retain UES to provide inspection services during the site preparation procedures for confirmation of the adequacy of the earthwork operations. Field tests and observations include verification of foundation and pavement subgrades by monitoring earthwork operations and performing quality assurance tests of the placement of compacted structural fill courses.

The geotechnical engineering design does not end with the advertisement of the construction documents. The design is an on-going process throughout construction. Because of our familiarity with the site conditions and the intent of the engineering design, we are most qualified to address site problems or construction changes, which may arise during construction, in a timely and cost-effective manner.

16.0 LIMITATIONS

This report has been prepared for the exclusive use of ***Pace Road DC Development, LLC*** and other designated members of their design/construction team associated with the proposed construction for the specific project discussed in this report. No other site or project facilities should be designed using the soil information contained in this report. As such, UES will not be responsible for the performance of any other site improvement designed using the data in this report.

This report should not be relied upon for final design recommendations or professional opinions by unauthorized third parties without the expressed written consent of UES. Unauthorized third parties that rely upon the information contained herein without the expressed written consent of UES assume all risk and liability for such reliance.

The recommendations submitted in this report are based upon the data obtained from the soil borings performed at the locations indicated on the Boring Location Plan and from other information as referenced. This report does not reflect any variations which may occur between the boring locations. The nature and extent of such variations may not become evident until the course of construction. If variations become evident, it will then be necessary for a re-evaluation of the recommendations of this report after performing on-site observations during the construction period and noting the characteristics of the variations.

Borings for a typical geotechnical report are widely spaced and generally not sufficient for reliably detecting the presence of isolated, anomalous surface or subsurface conditions, or reliably estimating unsuitable or suitable material quantities. Accordingly, UES does not recommend relying on our boring information for estimation of material quantities unless our contracted services ***specifically*** include sufficient exploration for such purpose(s) and within the report we so state that the level of exploration provided should be sufficient to detect anomalous conditions or estimate such quantities. Therefore, UES will not be responsible for any extrapolation or use of our data by others beyond the purpose(s) for which it is applicable or intended.

All users of this report are cautioned that there was no requirement for UES to attempt to locate any man-made buried objects or identify any other potentially hazardous conditions that may exist at the site during the course of this exploration. Therefore, no attempt was made by UES to locate or identify such concerns. UES cannot be responsible for any buried man-made objects or environmental hazards which may be subsequently encountered

during construction that are not discussed within the text of this report. We can provide this service if requested.

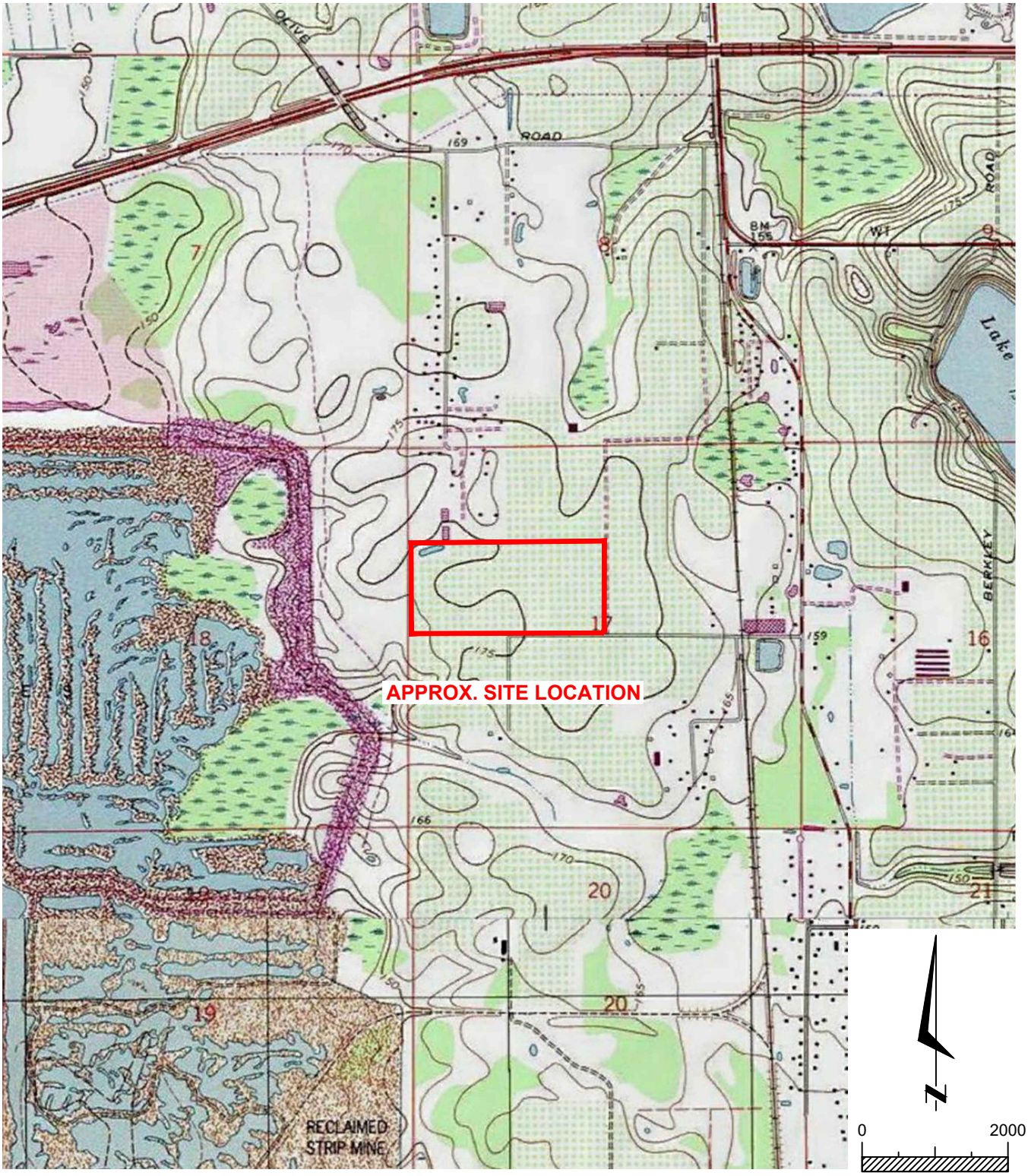
During the early stages of most construction projects, geotechnical issues not addressed in this report may arise. Because of the natural limitations inherent in working with the subsurface, it is not possible for a geotechnical engineer to predict and address all possible problems. Geotechnical Business Council (GBC) publication, "Important Information About This Geotechnical Engineering Report" appears in Appendix, and will help explain the nature of geotechnical issues.

Further, we present documents in Appendix: Constraints and Restrictions, to bring to your attention the potential concerns and the basic limitations of a typical geotechnical report.

* * * * * * * * *

APPENDIX A

UES™



SOURCE: USGS QUADRANGLE MAP OF "POLK CITY, FLORIDA".

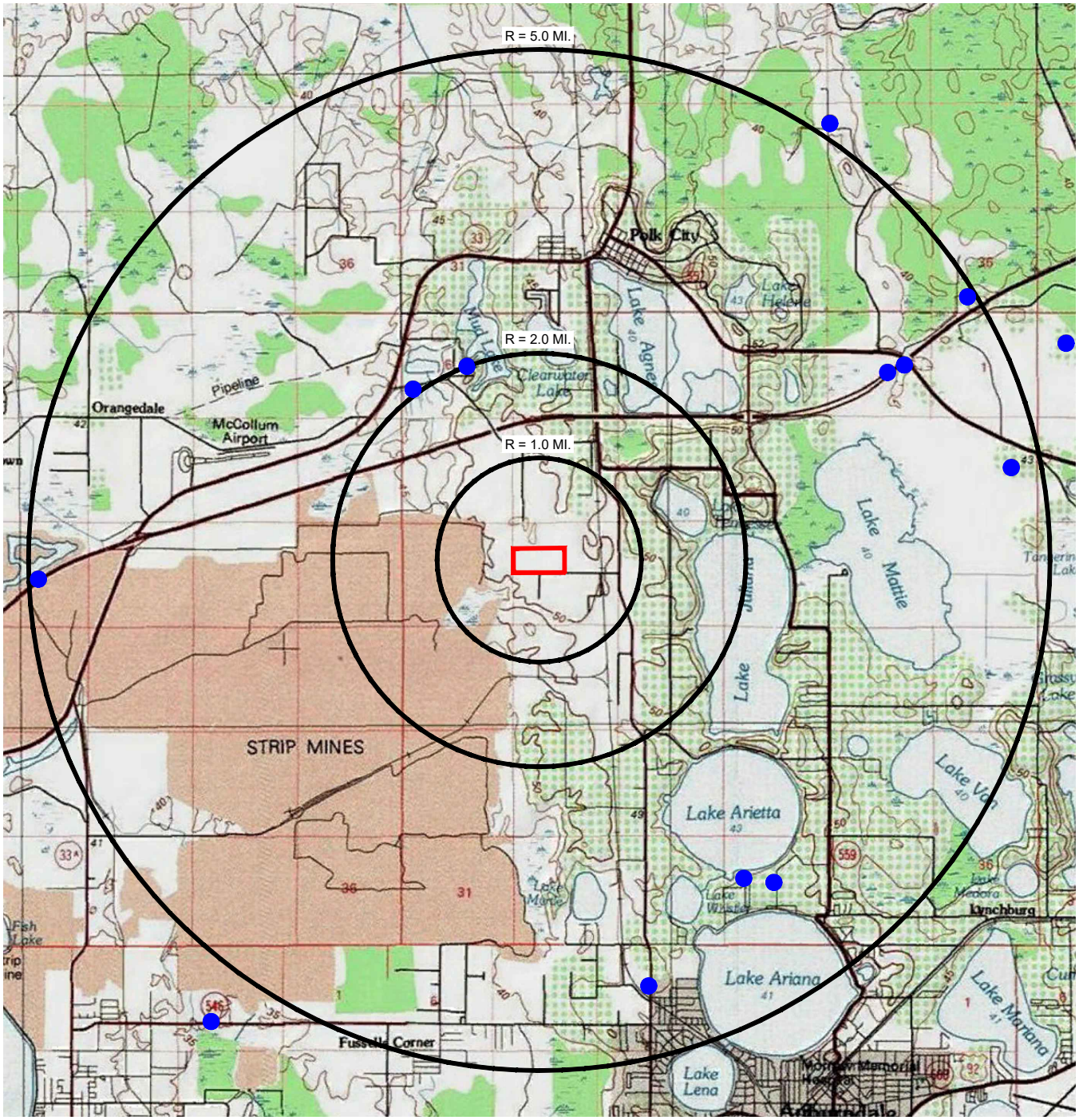
UES

GEOTECHNICAL EXPLORATION
PACE ROAD INDUSTRIAL DEVELOPMENT
AUBURNDALE, POLK COUNTY, FLORIDA

SITE LOCATION MAP

DRAWN BY:	N.F.	**DATE:**	3 - 9 - 2026	**CHECKED BY:**	Z.W.A.	**DATE:**	3 - 23 - 2026
SCALE:	AS SHOWN	**PROJECT NO:**	A26139.00190.001	**REPORT NO:**	2163464	**PAGE NO:**	A-1

26-0089-01



GEOTECHNICAL EXPLORATION
PACE ROAD INDUSTRIAL DEVELOPMENT
AUBURNDALE, POLK COUNTY, FLORIDA

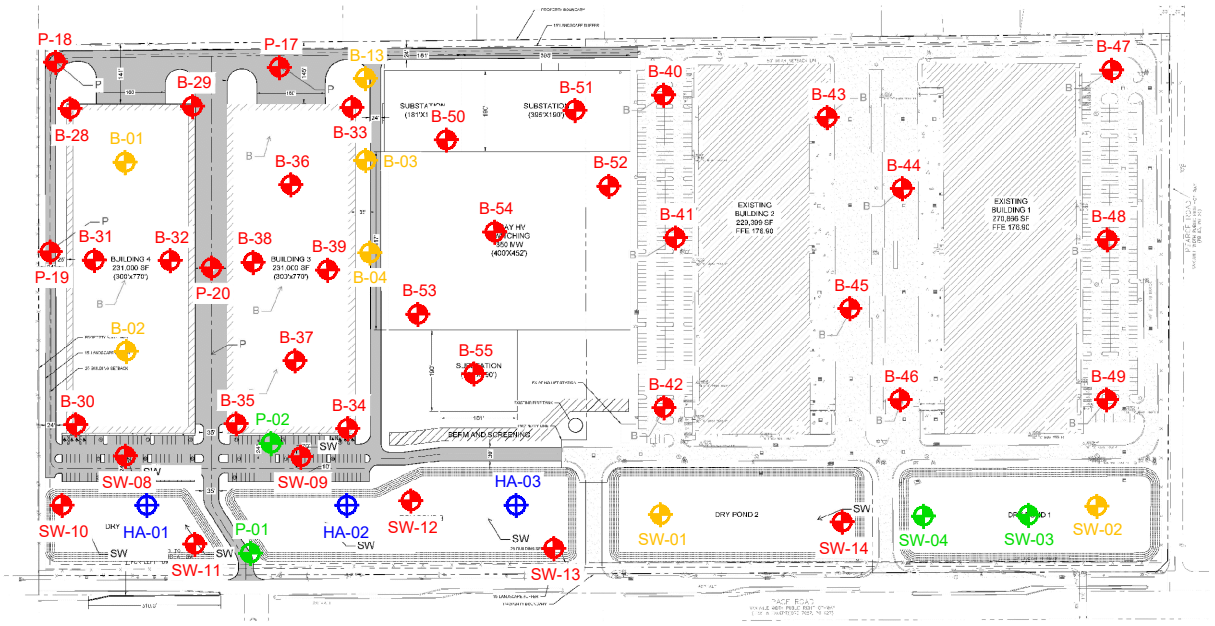
MAP OF FGS RECORDED SUBSIDENCE LOCATIONS

DRAWN BY:	N.F.	DATE:	3 - 9 - 2026	CHECKED BY:	Z.W.A.	DATE:	3 - 23 - 2026
SCALE:	AS SHOWN	PROJECT NO:	A26139.00190.001	REPORT NO:	2163464	PAGE NO:	A-2

26-0089-01

APPENDIX B

UES™



LEGEND

- APPROX. STANDARD PENETRATION TEST BORING LOCATION (SPT) PERFORMED 3/6/2026 - 3/18/2026
- PREVIOUS STANDARD PENETRATION TEST BORING LOCATION (SPT) PERFORMED 4/11/2024 - 4/22/2024
- PREVIOUS STANDARD PENETRATION TEST BORING LOCATION (SPT) PERFORMED 5/25/2022 - 5/27/2022
- PREVIOUS HAND AUGER BORING LOCATION PERFORMED 10/13/2022

0 450

SCALE (FT.)

N

THIS DRAWING CREATED USING PLAN PROVIDED BY CLIENT.

UES

SUPPLEMENTAL GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING LOCATION PLAN

DRAWN BY:	N.F.	**DATE:**	3 - 9 - 2026	**CHECKED BY:**	Z.W.A.	**DATE:**	3 - 23 - 2026
SCALE:	AS SHOWN	**PROJECT NO:**	A26139.00190.001	**REPORT NO:**	2163464	**PAGE NO:**	B-1

24-0328-02

BORING NO: B-28

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/16/2026 **DATE COMPLETED** 03/16/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/16/2026 **EST. S.H.G.W.T (FT.)** 0'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan Clayey fine **SAND** [**SC**]						
		6-7-7	14									
		5-5-5	10				18	9				
						4						
						Loose grey brown fine **SAND** [**SP**]						
5		3-4-5	9									
		2-3-3	6									
						7						
		3-4-7	11			Medium dense orange brown Clayey fine **SAND** [**SC**]						
		8-13-15	28									
10												
						Grey tan						
15		7-8-10	18									
20		9-8-8	16									
						23.5						
		10-9-7	16			Medium dense grey brown fine **SAND** with silt [**SP-SM**]						
25												

FIGURE NUMBER: B-2

LOG NAME: B-28

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						28.5						
30		8-11-10	21			Medium dense grey brown Silty fine **SAND** [**SM**]						
35		6-5-5	10			Loose						
40		4-3-4	7			43.5						
45		8-6-6	12			Stiff grey tan **LEAN CLAY** [**CL**]						
50		9-11-13	24			Very stiff 50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-28

BORING NO: B-29

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/16/2026 **DATE COMPLETED** 03/16/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/16/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		3-3-3	6									
		3-4-4	8	▽								
						4						
						Medium dense grey tan Clayey fine **SAND** [**SC**]						
5		4-4-9	13									
		8-9-14	23									
		15-15-16	31			Dense						
		14-17-19	36									
10												
		9-8-6	14			Medium dense	30	23				
15												
		8-7-6	13									
20												
						23.5						
		8-7-7	14			Medium dense grey tan Silty fine **SAND** [**SM**]						
25												

FIGURE NUMBER: B-2

LOG NAME: B-29

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		6-5-6	11									
35		5-4-5	9			Loose						
40		9-11-13	24			Medium dense						
						43.5						
45		7-10-8	18			Medium dense grey tan Silty Clayey fine **SAND** [**SC-SM**]						
50		10-11-10	21			50	35	80				
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-29

BORING NO: B-30

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/10/2026 **DATE COMPLETED** 03/10/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/10/2026 **EST. S.H.G.W.T (FT.)** 15'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND**, with roots [**SP**]						
		5-3-4	7									
		3-3-4	7			Grey tan						
5		3-3-3	6									
		3-4-5	9									
		4-4-5	9			8.5						
		5-5-6	11			Medium dense grey tan fine **SAND** with clay [**SP-SC**]						
10												
		9-8-10	18	▽								
15												
						18.5						
		7-8-8	16			Medium dense grey Silty fine **SAND** [**SM**]						
20												
						Loose						
		4-4-5	9									
25												

FIGURE NUMBER: B-2

LOG NAME: B-30

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		3-4-3	7									
35		3-3-4	7									
						38.5						
40		4-4-5	9			Loose grey tan Clayey fine **SAND** [**SC**]	28	91				
45		15-17-19	36			Dense, tan grey						
50		42-50/2"	50			Very dense, grey, with shell						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-30

BORING NO: B-31

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/13/2026 **DATE COMPLETED** 03/13/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/13/2026 **EST. S.H.G.W.T (FT.)** 8'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey brown fine **SAND** [**SP**]						
		2-2-2	4									
		2-2-2	4									
5		2-2-3	5									
		3-6-7	13			Medium dense						
						7						
		5-7-8	15	▽		Medium dense tan fine **SAND** with clay [**SP-SC**]						
						8.5						
		6-7-11	18			Medium dense tan Clayey fine **SAND** [**SC**]						
10												
15		9-12-14	26									
						18.5						
		12-13-15	28			Medium dense grey tan fine **SAND** with clay [**SP-SC**]						
20												
						23.5						
		10-13-14	27			Medium dense grey brown fine **SAND** [**SP**]						
25												

FIGURE NUMBER: B-2

LOG NAME: B-31

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						28.5						
30		2-2-3	5			Loose grey brown fine **SAND** with silt [**SP-SM**]	10	26				
						33.5						
35		3-3-2	5			Loose grey Silty fine **SAND** [**SM**]						
40		3-4-6	10									
45		4-5-6	11			Medium dense						
						48.5						
50		6-8-8	16			Medium dense grey tan Clayey fine **SAND** [**SC**]						
						50						
						End of Boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-31

BORING NO: B-32

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/13/2026 **DATE COMPLETED** 03/13/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/13/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey brown fine **SAND** [**SP**]						
		3-2-2	4									
		3-3-3	6									
5		3-4-5	9									
		5-9-10	19	▽		Medium dense						
						7						
		5-7-12	19			Medium dense grey tan Clayey fine **SAND** [**SC**]						
		10-13-13	26			Tan orange						
10												
		9-10-12	22			Grey tan						
15												
		10-10-13	23									
20												
		9-14-13	27									
25												

FIGURE NUMBER: B-2

LOG NAME: B-32

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						28.5						
30		1-1-1	2			Very loose grey brown Silty fine **SAND** [**SM**]						
35		2-3-3	6			Loose	18	27				
40		2-3-4	7			43.5						
45		4-5-7	12			Medium dense grey Clayey fine **SAND** [**SC**]						
50		5-8-9	17			Grey tan 50						
						End of Boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-32

BORING NO: B-33

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/11/2026 **DATE COMPLETED** 03/11/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		2-4-3	7			Brown grey						
		3-4-4	8			Grey						
						4						
5		3-14-17	31	▽		Dense grey fine **SAND** with clay, with cemented sand [**SP-SC**]						
						5.5						
		16-13-11	24			Medium dense grey Clayey fine **SAND**, with cemented sand [**SC**]	26	13				
		12-13-16	29									
		14-13-16	29			Grey tan						
10												
						13.5						
		9-10-11	21			Medium dense grey Silty fine **SAND** [**SM**]						
15												
20		4-7-5	12									
		3-6-7	13									
25												

FIGURE NUMBER: B-2

LOG NAME: B-33

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		5-5-6	11									
35		3-4-4	8			Loose						
40		4-5-4	9									
45		3-5-8	13			Medium dense, grey tan						
						48.5						
50		9-12-19	31			Dense grey tan Clayey fine **SAND** [**SC**]						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-33

BORING NO: B-34

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/09/2026 **DATE COMPLETED** 03/09/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/09/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND** [**SP**]						
		2-3-2	5									
		2-3-3	6			Grey						
5		5-7-6	13	▽		Medium dense, grey tan						
						5.5						
		5-6-11	17			Medium dense grey orange tan Clayey fine **SAND** [**SC**]	37	19				
		12-13-13	26			Grey tan						
		12-14-15	29									
10												
						13.5						
		7-7-8	15			Medium dense grey tan fine **SAND** with silt [**SP-SM**]						
15												
						18.5						
		5-6-6	12			Medium dense grey tan Silty fine **SAND** [**SM**]						
20												
		3-3-6	9			Loose, grey						
25												

FIGURE NUMBER: B-2

LOG NAME: B-34

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						28.5						
30		3-3-2	5			Loose grey fine **SAND** with silt, with cement pieces [**SP-SM**]						
35		2-2-3	5			38.5						
40		3-4-4	8			Loose grey tan Silty fine **SAND** [**SM**]						
45		13-14-15	29			43.5 Medium dense grey tan Clayey fine **SAND** [**SC**]						
50		17-43-50/3"	93			Very dense, grey, with shell 50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-34

BORING NO: B-35

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/09/2026 **DATE COMPLETED** 03/09/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/09/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-3-3	6									
		2-3-3	6			Grey tan						
						4						
5		2-3-6	9	▽		Loose grey tan fine **SAND** with clay [**SP-SC**]						
						5.5						
		7-9-9	18			Medium dense grey tan Clayey fine **SAND** [**SC**]						
		9-12-13	25									
		13-15-17	32			Dense						
10												
						13.5						
		9-10-12	22			Medium dense grey tan Silty fine **SAND** [**SM**]						
15												
		4-4-5	9			Loose						
20												
		4-5-6	11			Medium dense, grey						
25												

FIGURE NUMBER: B-2

LOG NAME: B-35

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		2-4-5	9			Loose						
35		3-3-3	6				13	24				
						38.5						
40		4-4-5	9			Loose grey tan fine **SAND** with silt [**SP-SM**]						
						43.5						
45		13-15-17	32			Dense grey tan Clayey fine **SAND** [**SC**]						
50		46-50/2"	50			Very dense, grey						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-35

BORING NO: B-36

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/11/2026 **DATE COMPLETED** 03/11/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		2-2-3	5									
		2-3-3	6			Brown grey Grey						
						4						
5		2-3-2	5			Loose grey fine **SAND** with silt [**SP-SM**]						
		3-4-6	10	▽		7						
		3-5-9	14			Medium dense grey tan Clayey fine **SAND** [**SC**]						
10		11-12-14	26			13.5						
		10-12-15	27			Medium dense grey Silty fine **SAND** [**SM**]						
15												
		7-6-8	14				19	20				
20												
		3-3-4	7			Loose						
25												

FIGURE NUMBER: B-2

LOG NAME: B-36

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		4-5-4	9									
35		3-3-4	7									
40		3-4-6	10			with shell						
45		3-4-4	8			Grey tan						
						48.5						
50		11-13-21	34			Dense grey tan Clayey fine **SAND** [**SC**]						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-36

BORING NO: B-37

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/10/2026 **DATE COMPLETED** 03/10/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/10/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-3-3	6									
		4-6-5	11	▽		Medium dense, grey						
						4						
		4-4-7	11			Medium dense grey orange tan Clayey fine **SAND**, with shell [**SC**]						
5		9-11-12	23			Grey tan						
		8-10-13	23									
		10-10-14	24									
10												
						13.5						
		8-9-9	18			Medium dense grey tan fine **SAND** with clay [**SP-SC**]						
15												
						18.5						
		9-10-11	21			Medium dense grey tan Silty fine **SAND** [**SM**]						
20												
		5-6-5	11			Grey						
25												

FIGURE NUMBER: B-2

LOG NAME: B-37

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		4-4-5	9			Loose						
35		4-5-5	10									
						38.5						
40		4-4-4	8			Loose grey tan Clayey fine **SAND** [**SC**]						
45		11-20-22	42			Dense	44	102				
50		21-50/2"	50			Very dense, grey, with shell						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-37

BORING NO: B-38

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/12/2026 **DATE COMPLETED** 03/12/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/12/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-3-3	6									
		3-4-5	9			Grey						
5		4-4-5	9	▽		5.5						
		5-12-15	27			Medium dense grey orange tan Clayey fine **SAND** [**SC**]						
		13-13-16	29			Grey tan						
10		14-13-15	28			13.5						
		7-8-9	17			Medium dense grey tan Silty fine **SAND** [**SM**]						
15												
		6-9-7	16			Grey						
20												
		5-5-6	11									
25												

FIGURE NUMBER: B-2

LOG NAME: B-38

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		5-6-6	12									
35		5-4-4	8			Loose						
40		6-5-5	10									
						43.5						
45		4-4-5	9			Loose grey tan Clayey fine **SAND** [**SC**]						
50		11-15-21	36			Dense	13	35				
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-38

BORING NO: B-39

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/12/2026 **DATE COMPLETED** 03/12/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/12/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		3-3-3	6			Brown grey, with roots						
		3-4-5	9	▽		Grey						
						4						
		4-6-9	15			Medium dense grey tan Clayey fine **SAND** [**SC**]						
5		11-14-16	30									
		16-15-15	30				27	13				
		11-10-13	23									
10												
						13.5						
		8-9-10	19			Medium dense grey Silty fine **SAND** [**SM**]						
15												
		5-8-6	14									
20												
		4-5-9	14									
25												

FIGURE NUMBER: B-2

LOG NAME: B-39

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
30		6-7-8	15									
35		4-5-4	9			Loose						
40		5-5-5	10									
45		3-7-8	15			Medium dense, with cement & shell						
						48.5						
50		10-14-20	34			Dense grey tan Clayey fine **SAND** [**SC**]						
						50						
						End of boring at 50'						

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-39

BORING NO: B-40

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/17/2026 **DATE COMPLETED** 03/17/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/17/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense brown fine **SAND** [**SP**]						
		3-8-12	20									
		8-8-7	15									
5		4-4-4	8			Loose						
		4-5-6	11			Medium dense						
		4-5-4	9			Loose						
						8.5						
		3-4-3	7			Loose tan fine **SAND** with clay [**SP-SC**]						
10				▽								
						13.5						
		3-9-9	18			Medium dense grey tan Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-40

BORING NO: B-41

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/17/2026 **DATE COMPLETED** 03/17/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/17/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense brown fine **SAND** [**SP**]						
		7-7-12	19									
		10-12-11	23									
5		4-5-6	11			Grey tan						
		4-6-6	12									
		5-5-7	12									
		6-9-10	19	▽								
10												
						13.5						
		6-7-7	14			Medium dense grey tan Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-41

BORING NO: B-42

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/17/2026 **DATE COMPLETED** 03/17/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) 12.5' **DATE** 03/17/2026 **EST. S.H.G.W.T (FT.)** 9.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense brown fine **SAND** [**SP**]						
		3-7-11	18									
		10-8-7	15									
5		6-12-14	26			Grey tan						
		14-12-11	23									
		6-6-7	13									
10		4-4-4	8	▽		Loose						
				▼		13.5						
		4-6-8	14			Medium dense grey brown Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-42

BORING NO: B-43

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/18/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense brown fine **SAND** [**SP**]						
		7-11-12	23									
		8-6-8	14			Grey brown						
5		4-5-5	10			Loose						
		6-6-8	14			Medium dense						
		5-6-5	11			8.5						
		4-4-5	9	▽		Loose grey brown fine **SAND** with clay [**SP-SC**]						
10						13.5						
		10-12-13	25			Medium dense grey brown Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-43

BORING NO: B-44

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/17/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan fine **SAND** [**SP**]						
		11-12-12	24									
		6-6-7	13									
5		6-6-6	12									
		6-6-7	13			7						
		4-4-5	9			Loose grey brown fine **SAND** with clay [**SP-SC**]						
		5-6-7	13			Medium dense						
10				▽								
						13.5						
		6-7-12	19			Medium dense grey brown Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-44

BORING NO: B-45

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/18/2026 **EST. S.H.G.W.T (FT.)** 8'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan fine **SAND** [**SP**]						
		10-13-16	29									
		14-15-16	31									
5		7-8-8	16									
		4-4-5	9			Loose, grey tan						
		5-5-7	12	▽		Medium dense						
						8.5						
		5-5-5	10			Loose grey brown Clayey fine **SAND** [**SC**]						
10												
		11-12-9	21			Medium dense						
						15						
15						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-45

BORING NO: B-46

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/17/2026 **DATE COMPLETED** 03/17/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) 13' **DATE** 03/17/2026 **EST. S.H.G.W.T (FT.)** 10'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense grey tan fine **SAND** [**SP**]						
		9-11-12	23									
		12-14-13	27									
5		6-6-7	13									
		4-3-4	7			Loose						
		4-5-7	12			Medium dense						
						8.5						
		4-5-5	10	▽		Loose grey tan fine **SAND** with clay [**SP-SC**]						
10												
				▼		13.5						
		5-7-13	20			Medium dense brown Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-46

BORING NO: B-47

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/18/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense brown fine **SAND** with silt [**SP-SM**]						
		5-5-6	11									
		4-8-10	18			4						
5		11-8-7	15			Medium dense grey tan fine **SAND** [**SP**]						
		4-4-3	7			Loose						
		3-3-3	6									
		3-2-3	5									
10				▽								
						13.5						
		5-5-5	10			Loose brown Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-47

BORING NO: B-48

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/18/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan fine **SAND** [**SP**]						
		10-12-16	28									
		13-14-9	23									
5		7-5-6	11									
		6-6-7	13									
		5-5-6	11			Grey brown						
		6-6-7	13	▽								
10						13.5						
		5-11-14	25			Medium dense dark brown fine **SAND** with silt [**SP-SM**] 15						
15						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-48

BORING NO: B-49

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/18/2026 **DATE COMPLETED** 03/18/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/18/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan fine **SAND** [**SP**]						
		2-5-6	11									
		6-8-7	15									
5		3-3-4	7			Loose						
		4-5-5	10			Grey brown						
		3-4-5	9									
		6-6-7	13	▽		Medium dense						
10												
						13.5						
		8-14-17	31			Dense dark brown fine **SAND** with silt [**SP-SM**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-49

BORING NO: B-50

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/13/2026 **DATE COMPLETED** 03/13/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/13/2026 **EST. S.H.G.W.T (FT.)** 10'+

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		2-3-3	6									
		3-4-3	7			Grey brown						
5		2-3-3	6									
		3-4-5	9				5	3				
		4-3-4	7			8.5						
		4-6-11	17	▽		Medium dense grey brown fine **SAND** with clay [**SP-SC**]						
10						13.5						
		10-13-11	24			Medium dense grey Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-50

BORING NO: B-51

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/14/2026 **DATE COMPLETED** 03/14/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/14/2026 **EST. S.H.G.W.T (FT.)** 8'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
		2-2-2	4			Loose grey brown fine **SAND** [**SP**]						
		2-2-2	4			4						
5		1-1-2	3			Very loose grey tan fine **SAND** with clay [**SP-SC**]	6	4				
		3-3-4	7			Loose						
		3-3-3	6	▽		8.5						
10		3-4-6	10			Loose grey tan Clayey fine **SAND** [**SC**]						
15		9-14-17	31			Dense						
						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-51

BORING NO: B-52

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/14/2026 **DATE COMPLETED** 03/14/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/14/2026 **EST. S.H.G.W.T (FT.)** 8'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
		4-5-6	11			Medium dense brown fine **SAND** [**SP**]						
						2.5						
		4-6-6	12			Medium dense brown fine **SAND** with clay [**SP-SC**]	8	4				
						4						
5		3-3-3	6			Loose grey brown fine **SAND** [**SP**]						
		3-5-5	10									
		4-3-4	7	▽								
						8.5						
		4-7-9	16			Medium dense grey tan Clayey fine **SAND** [**SC**]						
10												
		8-13-13	26									
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-52

BORING NO: B-53

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/11/2026 **DATE COMPLETED** 03/11/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey fine **SAND** [**SP**]						
		3-4-4	8									
		3-4-3	7	▽		4						
5		4-8-14	22			Medium dense grey orange tan Clayey fine **SAND** [**SC**]	46	21				
		12-14-16	30			Grey tan						
		10-11-12	23			8.5						
		8-7-7	14			Medium dense grey fine **SAND** with clay [**SP-SC**]						
10						13.5						
		13-14-15	29			Medium dense grey Clayey fine **SAND** [**SC**]						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-53

BORING NO: B-54

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/12/2026 **DATE COMPLETED** 03/12/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/12/2026 **EST. S.H.G.W.T (FT.)** 8'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		4-4-4	8									
		4-7-7	14			Medium dense, grey brown						
5		4-4-5	9			Loose						
		4-5-7	12			Medium dense						
						7						
		5-6-6	12	▽		Medium dense grey brown fine **SAND** with clay [**SP-SC**]						
						8.5						
		5-7-8	15			Medium dense grey tan Clayey fine **SAND** [**SC**]	25	14				
10												
		9-12-14	26									
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-54

BORING NO: B-55

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/10/2026 **DATE COMPLETED** 03/10/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) NE **DATE** 03/10/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-3-4	7									
		3-4-4	8	▽		Grey						
						4						
		2-3-4	7			Loose grey tan Clayey fine **SAND** [**SC**]	25	13				
5		11-23-29	52			Very dense, grey brown						
		18-22-31	53									
		20-24-33	57			Grey orange						
10												
		12-12-14	26			Medium dense, grey tan						
15						15						
						End of boring at 15'						
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: B-55

BORING NO: P-17

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/14/2026 **DATE COMPLETED** 03/14/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/14/2026 **EST. S.H.G.W.T (FT.)** 3.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		2-3-4	7									
		4-5-4	9	▽		4						
		5-8-10	18			Medium dense grey tan Clayey fine **SAND** [**SC**]						
5		12-15-16	31			Dense						
		11-14-18	32									
		13-13-15	28			10						
10						End of boring at 10'						
15												
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: P-17

BORING NO: P-18

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/14/2026 **DATE COMPLETED** 03/14/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** CM/CA

WATER TABLE (FT.) NE **DATE** 03/14/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown fine **SAND** [**SP**]						
		2-2-2	4									
		2-3-3	6									
5		2-3-3	6			Grey brown						
		5-6-7	13	▽		Medium dense						
						7						
		7-9-13	22			Medium dense grey tan Clayey fine **SAND** [**SC**]						
		12-13-15	28			10						
10						End of boring at 10'						
15												
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: P-18

BORING NO: P-19

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/07/2026 **DATE COMPLETED** 03/07/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/IM / ZWA

WATER TABLE (FT.) NE **DATE** 03/07/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-2-3	5									
		3-3-4	7			Grey						
5		3-4-5	9	▽		Tan 5.5						
		4-5-7	12			Medium dense tan grey Clayey fine **SAND** [**SC**]						
		9-10-12	22			Grey tan						
		10-14-17	31			Dense 10						
10						End of boring at 10'						
15												
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: P-19

BORING NO: P-20

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/07/2026 **DATE COMPLETED** 03/07/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/IM / ZWA

WATER TABLE (FT.) NE **DATE** 03/07/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS NE = NOT ENCOUNTERED

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey fine **SAND** [**SP**]						
		3-3-3	6									
		3-3-4	7									
5		3-4-4	8									
		4-5-8	13	▽		Medium dense, tan						
						7						
		7-8-7	15			Medium dense grey orange tan Clayey fine **SAND** [**SC**]						
		11-12-14	26			Grey tan						
10						10						
						End of boring at 10'						
15												
20												
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: P-20

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.15

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **SW-01** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/25/2022
EST. SHGWT (ft): 13.0

DATE STARTED: 5/25/22
DATE FINISHED: 5/25/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose grey tan fine SAND [SP]						
		3-2-2	4									
		2-3-3	6									
5		2-2-2	4			Loose tan fine SAND with clay [SP-SC]	6	5			16	
		2-2-2	4			Loose tan fine SAND [SP]						
		3-2-3	5									
10		2-2-2	4									
				▽								
						Medium dense dark grey tan clayey fine SAND [SC]	32	16				
15		6-9-12	21									
						-- dark grey						
20		8-10-10	20									
						-- grey						
25		7-7-8	15			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.16

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **SW-02** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/25/2022
EST. SHGWT (ft): 13.0

DATE STARTED: 5/25/22
DATE FINISHED: 5/25/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose grey fine SAND with roots [SP]						
		2-3-3	6									
		3-2-2	4			-- very loose, light grey						
5		1-1-1	2			-- loose	5	3			22	
		1-2-3	5									
		2-2-3	5									
		3-4-5	9									
10				▽								
						Medium dense dark grey tan clayey fine SAND [SC]						
15		7-10-11	21									
							19	16				
20		9-9-10	19									
						-- grey						
25		8-8-9	17			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UES

UES
BORING LOG

PROJECT NO.:	0130.2200200.0001
REPORT NO.:	2086203
PAGE:	B-2.31

PROJECT: SUPPLEMENTAL GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **SW-03** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOLN PROPERTY COMPANY

LOCATION: SEE BORING LOCATION PLAN

REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): N.E.
DATE OF READING: 4/16/2024
EST. SHGWT (ft): 10.0+

DATE STARTED: 4/16/24
DATE FINISHED: 4/16/24
DRILLED BY: PURA VIDA
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0												
		2-2-2-1	4			Loose brown fine SAND with clay [SP-SC]	6	2			19	
		1-1-1-1	2			Very loose light brown fine SAND [SP]						
5		1-2-1-1	3				5	3			23	
		2-1-2-2	3			Loose light brown fine SAND with clay [SP-SC]						
10		2-2-2-2	4	▽								
		4-7-8	15			Medium dense light brown clayey fine SAND [SC]						
15						BORING TERMINATED AT 15.0 FEET						
20												
25												
30												

W-13473.GPJ

UES
BORING LOG

PROJECT NO.:	0130.2200200.0001
REPORT NO.:	2086203
PAGE:	B-2.32

PROJECT: SUPPLEMENTAL GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **SW-04** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOLN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): N.E.
DATE OF READING: 4/16/2024
EST. SHGWT (ft): 10.0+

DATE STARTED: 4/16/24
DATE FINISHED: 4/16/24
DRILLED BY: PURA VIDA
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose brown fine SAND [SP]						
		2-2-2-2	4			-- very loose						
		1-1-WH-1	1			Very loose light brown fine SAND with clay [SP-SC]	6	2			17	
5		1-1-1-1	2			-- loose						
		1-2-2-2	4				9	6			9	
		2-2-3-2	5	▽								
10												
						-- medium dense						
		5-6-7	13									
15						BORING TERMINATED AT 15.0 FEET						
20												
25												
30												

W-13473.GPJ

BORING NO: SW-08

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/07/2026 **DATE COMPLETED** 03/07/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/IM / ZWA

WATER TABLE (FT.) 15' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 12'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey tan fine **SAND** [**SP**]						
		3-3-3	6									
		3-2-3	5									
5		2-2-2	4			Grey orange tan						
		3-4-5	9									
		3-4-4	8				3	3				38
		4-6-6	12			Medium dense, grey						
10				▽								
						13.5						
		5-7-8	15	▼		Medium dense grey tan Clayey fine **SAND** [**SC**]	16	25				
15												
		4-4-5	9			Loose						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-08

BORING NO: SW-09

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/06/2026 **DATE COMPLETED** 03/06/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) 15.5' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 5'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey fine **SAND** [**SP**]						
		3-3-3	6									
		3-4-4	8				5	3				24
5		3-4-7	11	▽		Medium dense						
						5.5						
		8-11-14	25			Medium dense grey orange tan Clayey fine **SAND** [**SC**]	33	15				
		12-14-16	30			Grey tan						
		14-15-17	32			Dense						
10												
		10-8-11	19	▼		Medium dense						
15												
						18.5						
		11-13-13	26			Medium dense grey fine **SAND** with clay [**SP-SC**]						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-09

BORING NO: SW-10

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/07/2026 **DATE COMPLETED** 03/07/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/IM / ZWA

WATER TABLE (FT.) 15.5' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 12.5'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND**, with roots [**SP**]						
		2-2-2	4									
		2-2-2	4			Grey tan						
5		3-2-3	5				5	4				20
		3-4-5	9									
		3-4-4	8			8.5						
		5-7-8	15			Medium dense grey tan fine **SAND** with clay [**SP-SC**]						
10				▽								
		4-3-4	7			Loose, grey	10	18				
15				▼								
						18.5						
		4-6-7	13			Medium dense grey Clayey fine **SAND** [**SC**]						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-10

BORING NO: SW-11

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/06/2026 **DATE COMPLETED** 03/06/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) 15.5' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose grey tan fine **SAND** [**SP**]						
		3-5-5	10									
		3-5-4	9			Tan						
5		2-4-3	7									
		3-5-6	11	▽		Medium dense	5	4				24
						7						
		7-9-10	19			Medium dense grey orange tan Clayey fine **SAND** [**SC**]	31	17				
		10-12-14	26			Grey tan						
10												
						13.5						
		10-8-8	16			Medium dense grey Silty fine **SAND** [**SM**]						
15				▼								
		5-6-9	15									
						20						
20						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-11

BORING NO: SW-12

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/06/2026 **DATE COMPLETED** 03/06/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) 14.5' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND** [**SP**]						
		3-4-5	9									
		4-4-5	9			Grey						
5		3-3-5	8				4	3				25
		3-5-8	13	▽		Medium dense						
						7						
		6-8-9	17			Medium dense grey orange tan Clayey fine **SAND** [**SC**]	37	20				
		9-12-11	23			Grey tan						
10												
		11-13-15	28	▼		Grey						
15												
		6-9-13	22			Grey tan						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-12

BORING NO: SW-13

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/06/2026 **DATE COMPLETED** 03/06/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) 14' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 6.5'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Loose brown grey fine **SAND** [**SP**]						
		3-3-4	7									
		4-7-8	15			Medium dense, grey						
						4						
5		5-6-7	13			Medium dense grey tan fine **SAND** with clay [**SP-SC**]	9	4				9
		7-7-6	13	▽		7						
		5-6-8	14			Medium dense grey tan Clayey fine **SAND** [**SC**]						
		9-9-12	21				28	14				
10												
						13.5						
		5-7-7	14	▼		Medium dense grey Silty fine **SAND** [**SM**]						
15												
						18.5						
		8-10-10	20			Medium dense grey Clayey fine **SAND** [**SC**]						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-13

BORING NO: SW-14

PROJECT PACE ROAD INDUSTRIAL DEVELOPMENT

PROJECT ADDRESS PACE ROAD, AUBURNDALE, POLK COUNTY, FL

CLIENT NAME PACE ROAD DCDEVELOPMENT, LLC **PROJECT NO** A26139.00190.001 **REPORT NO.**

LOCATION PACE ROAD, AUBURNDALE, POLK COUNTY, FL **DATE STARTED** 03/06/2026 **DATE COMPLETED** 03/06/2026

SAMPLE METHOD ADTM D-1586 **ELEV.** N/A **DRILLER/PM** ORL - CM/CA / ZWA

WATER TABLE (FT.) 20' **DATE** 03/11/2026 **EST. S.H.G.W.T (FT.)** 13'

COMMENTS -

DEPTH (FT)	SAMPLE TYPE	BLOW COUNTS/6"	N-VALUE	W.T.	SYMBOL	MATERIALS DESCRIPTION	-200 (%)	MC (%)	LL	PI	OC (%)	K (ft/day)
						Medium dense tan fine **SAND** [**SP**]						
		7-4-7	11									
		5-5-4	9			Loose						
5		3-3-4	7									
		3-2-3	5				5	3				21
		3-3-5	8									
		4-3-4	7			Grey						
10												
				▽		13.5						
		5-4-5	9			Loose grey Silty fine **SAND** [**SM**]	31	13				
15						18.5						
		9-12-11	23	▼		Medium dense grey Clayey fine **SAND** [**SC**]						
20						20						
						End of boring at 20'						
25												

REMARKS

FIGURE NUMBER: B-2

LOG NAME: SW-14

UES
BORING LOG

PROJECT NO.:	0130.2200200.0001
REPORT NO.:	2086203
PAGE:	B-2.15

PROJECT: SUPPLEMENTAL GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **P-01** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOLN PROPERTY COMPANY

LOCATION: SEE BORING LOCATION PLAN

REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): N.E.
DATE OF READING: 4/17/2024
EST. SHGWT (ft): 5.5

DATE STARTED: 4/17/24
DATE FINISHED: 4/17/24
DRILLED BY: PURA VIDA
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0												
		2-1-2-1	3			Very loose brown fine SAND [SP]						
		2-1-2-2	3			Very loose brown fine SAND with silt [SP-SM]						
5		2-1-2-3	3	▽								
		7-8-10-15	18			Medium dense light brown silty fine SAND [SM]						
10		12-11-12-13	23			BORING TERMINATED AT 10.0 FEET						
15												
20												
25												
30												

W-13473.GPJ

UES
BORING LOG

PROJECT NO.:	0130.2200200.0001
REPORT NO.:	2086203
PAGE:	B-2.16

PROJECT: SUPPLEMENTAL GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **P-02** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOLN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): N.E.
DATE OF READING: 4/17/2024
EST. SHGWT (ft): 5.5

DATE STARTED: 4/17/24
DATE FINISHED: 4/17/24
DRILLED BY: PURA VIDA
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0												
		2-1-1-2	2			Very loose brown fine SAND [SP]						
		2-1-2-1	3									
5						Loose brown fine SAND with silt [SP-SM]						
		2-2-3-3	5	▽								
						Medium dense brown silty fine SAND [SM]						
		10-8-10-13	18									
10		13-12-14-12	26			BORING TERMINATED AT 10.0 FEET						
15												
20												
25												
30												

W-13473.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.1

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **B-01** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/27/2022
EST. SHGWT (ft): 6.5

DATE STARTED: 5/27/22
DATE FINISHED: 5/27/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose dark grey fine SAND [SP]						
		2-3-3	6									
		4-3-4	7			-- light grey						
5		3-3-4	7			-- medium dense						
				▽								
		5-6-6	12			Medium dense grey clayey fine SAND [SC]	33	14				
		7-8-10	18			-- grey tan						
10		9-10-11	21									
15		8-8-9	17									
20		7-6-6	12									
						Medium dense light grey fine SAND with clay [SP-SC]						
25		9-9-10	19			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.2

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **B-02** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/27/2022
EST. SHGWT (ft): 6.5

DATE STARTED: 5/27/22
DATE FINISHED: 5/27/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose grey fine SAND [SP]						
		2-2-2	4									
		2-2-2	4			-- very loose, light grey						
5		2-1-2	3			-- loose						
		3-3-4	7	▽								
						Loose grey tan clayey fine SAND [SC]						
		3-3-4	7			-- medium dense						
10		10-11-14	25									
15		7-8-7	15									
						Medium dense grey tan silty fine SAND [SM]						
20		6-6-7	13									
						-- light grey						
25		8-9-8	17									
						BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.3

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **B-03** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/27/2022
EST. SHGWT (ft): 6.5

DATE STARTED: 5/27/22
DATE FINISHED: 5/27/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose dark grey fine SAND with roots [SP]						
		2-3-3	6			-- grey						
		3-3-3	6			-- light grey tan						
5		3-3-3	6	▽								
		4-4-6	10			Medium dense grey tan clayey fine SAND [SC]						
		6-8-9	17									
10		7-11-13	24									
15		9-12-14	26									
						-- dark grey						
20		10-13-15	28									
						-- grey						
25		8-9-11	20			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.4

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **B-04** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/27/2022
EST. SHGWT (ft): 3.5

DATE STARTED: 5/27/22
DATE FINISHED: 5/27/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose grey fine SAND [SP]						
		4-4-4	8									
						-- medium dense						
		4-6-5	11	▽								
						Medium dense grey tan clayey fine SAND [SC]						
5							28	12				
		5-7-8	15									
		9-12-14	26									
		7-11-16	27			-- dense						
10		14-16-18	34									
						-- medium dense, grey						
15		7-8-8	16									
						Loose grey fine SAND with clay [SP-SC]						
20		5-5-5	10									
						Medium dense grey clayey fine SAND [SC]						
25		6-8-9	17			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

UNIVERSAL ENGINEERING SCIENCES
BORING LOG

PROJECT NO.:	0130.2200200.0000
REPORT NO.:	1957237
PAGE:	B-2.13

PROJECT: PRELIMINARY GEOTECHNICAL EXPLORATION
AUBURNDALE WAREHOUSE FACILITY
POLK COUNTY, FLORIDA

BORING I.D.: **B-13** SHEET: **1 of 1**
SECTION: TOWNSHIP: RANGE:

CLIENT: LINCOIN PROPERTY COMPANY
LOCATION: SEE BORING LOCATION PLAN
REMARKS: SHGWT = SEASONAL HIGH GROUNDWATER TABLE, N.S. = NOT SURVEYED, NE = NOT ENCOUNTERED

G.S. ELEVATION (ft): N.S.
WATER TABLE (ft): NE
DATE OF READING: 5/26/2022
EST. SHGWT (ft): 6.5

DATE STARTED: 5/26/22
DATE FINISHED: 5/26/22
DRILLED BY: ORL - CM/FB
TYPE OF SAMPLING: ASTM D 1586

DEPTH (FT.)	SAMPLE	BLOWS PER 6" INCREMENT	N BLOWS / FT	W.T.	SYMBOL	DESCRIPTION	-200 (%)	MC (%)	ATTERBERG LIMITS LL	ATTERBERG LIMITS PI	K (FT/ DAY)	ORG. CONT. (%)
0						Loose dark tan fine SAND [SP]						
		4-4-3	7									
						Medium dense tan fine SAND with clay [SP-SC]	7	5				
		4-4-4	8									
5		4-6-10	16									
				▽								
		9-11-12	23			Medium dense grey tan clayey fine SAND [SC]						
		10-12-15	27									
10		13-13-12	25									
15		8-11-12	23									
20		6-8-8	16									
25		6-7-8	15			BORING TERMINATED AT 25.0 FEET						

W-12277.GPJ

KEY TO BORING LOGS

SYMBOLS AND ABBREVIATIONS

SYMBOL	DESCRIPTION
N-Value	No. of Blows of a 140-lb. Weight Falling 30 Inches Required to Drive a Standard Spoon 1 Foot
WOR	Weight of Drill Rods
WOH	Weight of Drill Rods and Hammer
	Sample from Auger Cuttings
	Standard Penetration Test Sample
	Thin-wall Shelby Tube Sample (Undisturbed Sampler Used)
RQD	Rock Quality Designation
	Stabilized Groundwater Level
	Seasonal High Groundwater Level (also referred to as the W.S.W.T.)
NE	Not Encountered
GNE	Groundwater Not Encountered
BT	Boring Terminated
-200 (%)	Fines Content or % Passing No. 200 Sieve
MC (%)	Moisture Content
LL	Liquid Limit (Atterberg Limits Test)
PI	Plasticity Index (Atterberg Limits Test)
NP	Non-Plastic (Atterberg Limits Test)
K	Coefficient of Permeability
Org. Cont.	Organic Content
G.S. Elevation	Ground Surface Elevation

UNIFIED SOIL CLASSIFICATION SYSTEM

MAJOR DIVISIONS			GROUP SYMBOLS	TYPICAL NAMES
COARSE GRAINED SOILS More than 50% retained on the No. 200 sieve*	GRAVELS 50% or more of coarse fraction retained on No. 4 sieve	CLEAN GRAVELS	GW	Well-graded gravels and gravel-sand mixtures, little or no fines
			GP	Poorly graded gravels and gravel-sand mixtures, little or no fines
		GRAVELS WITH FINES	GM	Silty gravels and gravel-sand-silt mixtures
			GC	Clayey gravels and gravel-sand-clay mixtures
	SANDS More than 50% of coarse fraction passes No. 4 sieve	CLEAN SANDS 5% or less passing No. 200 sieve	SW**	Well-graded sands and gravelly sands, little or no fines
			SP**	Poorly graded sands and gravelly sands, little or no fines
		SANDS with 12% or more passing No. 200 sieve	SM**	Silty sands, sand-silt mixtures
			SC**	Clayey sands, sand-clay mixtures
FINE-GRAINED SIOLS 50% or more passes the No. 200 sieve*	SILTS AND CLAYS Liquid limit 50% or less		ML	Inorganic silts, very fine sands, rock flour, silty or clayey fine sands
			CL	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays, lean clays
			OL	Organic silts and organic silty clays of low plasticity
	SILTS AND CLAYS Liquid limit greater than 50%		MH	Inorganic silts, micaceous or diamicaceous fine sands or silts, elastic silts
			CH	Inorganic clays or clays of high plasticity, fat clays
			OH	Organic clays of medium to high plasticity
			PT	Peat, muck and other highly organic soils

*Based on the material passing the 3-inch (75 mm) sieve
** Use dual symbol (such as SP-SM and SP-SC) for soils with more than 5% but less than 12% passing the No. 200 sieve

RELATIVE DENSITY
(Sands and Gravels)
Very loose – Less than 4 Blow/Foot
Loose – 4 to 10 Blows/Foot
Medium Dense – 11 to 30 Blows/Foot
Dense – 31 to 50 Blows/Foot
Very Dense – More than 50 Blows/Foot

CONSISTENCY
(Silts and Clays)
Very Soft – Less than 2 Blows/Foot
Soft – 2 to 4 Blows/Foot
Firm – 5 to 8 Blows/Foot
Stiff – 9 to 15 Blows/Foot
Very Stiff – 16 to 30 Blows/Foot
Hard – More than 30 Blows/Foot

RELATIVE HARDNESS
(Limestone)
Soft – 100 Blows for more than 2 Inches
Hard – 100 Blows for less than 2 Inches

MODIFIERS

These modifiers Provide Our Estimate of the Amount of Minor Constituents (Silt or Clay Size Particles) in the Soil Sample
Trace – 5% or less
With Silt or With Clay – 6% to 11%
Silty or Clayey – 12% to 30%
Very Silty or Very Clayey – 31% to 50%

These Modifiers Provide Our Estimate of the Amount of Organic Components in the Soil Sample
Trace – Less than 3%
Few – 3% to 4%
Some – 5% to 8%
Many – Greater than 8%

These Modifiers Provide Our Estimate of the Amount of Other Components (Shell, Gravel, Etc.) in the Soil Sample
Trace – 5% or less
Few – 6% to 12%
Some – 13% to 30%
Many – 31% to 50%

APPENDIX C

UES™

Important Information about This

Geotechnical-Engineering Report

Subsurface problems are a principal cause of construction delays, cost overruns, claims, and disputes.

While you cannot eliminate all such risks, you can manage them. The following information is provided to help.

Geotechnical Services Are Performed for Specific Purposes, Persons, and Projects

Geotechnical engineers structure their services to meet the specific needs of their clients. A geotechnical-engineering study conducted for a civil engineer may not fulfill the needs of a constructor — a construction contractor — or even another civil engineer. Because each geotechnical- engineering study is unique, each geotechnical-engineering report is unique, prepared *solely* for the client. No one except you should rely on this geotechnical-engineering report without first conferring with the geotechnical engineer who prepared it. *And no one — not even you* — should apply this report for any purpose or project except the one originally contemplated.

Read the Full Report

Serious problems have occurred because those relying on a geotechnical-engineering report did not read it all. Do not rely on an executive summary. Do not read selected elements only.

Geotechnical Engineers Base Each Report on a Unique Set of Project-Specific Factors

Geotechnical engineers consider many unique, project-specific factors when establishing the scope of a study. Typical factors include: the client's goals, objectives, and risk-management preferences; the general nature of the structure involved, its size, and configuration; the location of the structure on the site; and other planned or existing site improvements, such as access roads, parking lots, and underground utilities. Unless the geotechnical engineer who conducted the study specifically indicates otherwise, do not rely on a geotechnical-engineering report that was:

- not prepared for you;
- not prepared for your project;
- not prepared for the specific site explored; or
- completed before important project changes were made.

Typical changes that can erode the reliability of an existing geotechnical-engineering report include those that affect:

- the function of the proposed structure, as when it's changed from a parking garage to an office building, or from a light-industrial plant to a refrigerated warehouse;
- the elevation, configuration, location, orientation, or weight of the proposed structure;
- the composition of the design team; or
- project ownership.

As a general rule, *always* inform your geotechnical engineer of project changes—even minor ones—and request an assessment of their impact. *Geotechnical engineers cannot accept responsibility or liability for problems that occur because their reports do not consider developments of which they were not informed.*

Subsurface Conditions Can Change

A geotechnical-engineering report is based on conditions that existed at the time the geotechnical engineer performed the study. *Do not rely on a geotechnical-engineering report whose adequacy may have been affected by*: the passage of time; man-made events, such as construction on or adjacent to the site; or natural events, such as floods, droughts, earthquakes, or groundwater fluctuations. *Contact the geotechnical engineer before applying this report to determine if it is still reliable.* A minor amount of additional testing or analysis could prevent major problems.

Most Geotechnical Findings Are Professional Opinions

Site exploration identifies subsurface conditions only at those points where subsurface tests are conducted or samples are taken. Geotechnical engineers review field and laboratory data and then apply their professional judgment to render an opinion about subsurface conditions throughout the site. Actual subsurface conditions may differ — sometimes significantly — from those indicated in your report. Retaining the geotechnical engineer who developed your report to provide geotechnical-construction observation is the most effective method of managing the risks associated with unanticipated conditions.

A Report's Recommendations Are Not Final

Do not overrely on the confirmation-dependent recommendations included in your report. *Confirmation-dependent recommendations are not final,* because geotechnical engineers develop them principally from judgment and opinion. Geotechnical engineers can finalize their recommendations *only* by observing actual subsurface conditions revealed during construction. *The geotechnical engineer who developed your report cannot assume responsibility or liability for the report's confirmation-dependent recommendations if that engineer does not perform the geotechnical-construction observation required to confirm the recommendations' applicability.*

A Geotechnical-Engineering Report Is Subject to Misinterpretation

Other design-team members' misinterpretation of geotechnical-engineering reports has resulted in costly

problems. Confront that risk by having your geotechnical engineer confer with appropriate members of the design team after submitting the report. Also retain your geotechnical engineer to review pertinent elements of the design team's plans and specifications. Constructors can also misinterpret a geotechnical-engineering report. Confront that risk by having your geotechnical engineer participate in prebid and preconstruction conferences, and by providing geotechnical construction observation.

Do Not Redraw the Engineer's Logs

Geotechnical engineers prepare final boring and testing logs based upon their interpretation of field logs and laboratory data. To prevent errors or omissions, the logs included in a geotechnical-engineering report should *never* be redrawn for inclusion in architectural or other design drawings. Only photographic or electronic reproduction is acceptable, *but recognize that separating logs from the report can elevate risk.*

Give Constructors a Complete Report and Guidance

Some owners and design professionals mistakenly believe they can make constructors liable for unanticipated subsurface conditions by limiting what they provide for bid preparation. To help prevent costly problems, give constructors the complete geotechnical-engineering report, *but* preface it with a clearly written letter of transmittal. In that letter, advise constructors that the report was not prepared for purposes of bid development and that the report's accuracy is limited; encourage them to confer with the geotechnical engineer who prepared the report (a modest fee may be required) and/or to conduct additional study to obtain the specific types of information they need or prefer. A prebid conference can also be valuable. *Be sure constructors have sufficient time* to perform additional study. Only then might you be in a position to give constructors the best information available to you, while requiring them to at least share some of the financial responsibilities stemming from unanticipated conditions.

Read Responsibility Provisions Closely

Some clients, design professionals, and constructors fail to recognize that geotechnical engineering is far less exact than other engineering disciplines. This lack of understanding has created unrealistic expectations that have led to disappointments, claims, and disputes. To help reduce the risk of such outcomes, geotechnical engineers commonly include a variety of explanatory provisions in their reports. Sometimes labeled "limitations," many of these provisions indicate where geotechnical engineers' responsibilities begin and end, to help others recognize their own responsibilities and risks. *Read these provisions closely.* Ask questions. Your geotechnical engineer should respond fully and frankly.

Environmental Concerns Are Not Covered

The equipment, techniques, and personnel used to perform an *environmental* study differ significantly from those used to perform a *geotechnical* study. For that reason, a geotechnical-engineering report does not usually relate any environmental findings, conclusions, or recommendations; e.g., about the likelihood of encountering underground storage tanks or regulated contaminants. *Unanticipated environmental problems have led to numerous project failures.* If you have not yet obtained your own environmental information, ask your geotechnical consultant for risk-management guidance. *Do not rely on an environmental report prepared for someone else.*

Obtain Professional Assistance To Deal with Mold

Diverse strategies can be applied during building design, construction, operation, and maintenance to prevent significant amounts of mold from growing on indoor surfaces. To be effective, all such strategies should be devised for the *express purpose* of mold prevention, integrated into a comprehensive plan, and executed with diligent oversight by a professional mold-prevention consultant. Because just a small amount of water or moisture can lead to the development of severe mold infestations, many mold- prevention strategies focus on keeping building surfaces dry. While groundwater, water infiltration, and similar issues may have been addressed as part of the geotechnical- engineering study whose findings are conveyed in this report, the geotechnical engineer in charge of this project is not a mold prevention consultant; *none of the services performed in connection with the geotechnical engineer's study were designed or conducted for the purpose of mold prevention. Proper implementation of the recommendations conveyed in this report will not of itself be sufficient to prevent mold from growing in or on the structure involved.*

Rely, on Your GBC-Member Geotechnical Engineer for Additional Assistance

Membership in the Geotechnical Business Council of the Geoprofessional Business Association exposes geotechnical engineers to a wide array of risk-confrontation techniques that can be of genuine benefit for everyone involved with a construction project. Confer with you GBC-Member geotechnical engineer for more information.

GEOTECHNICAL BUSINESS COUNCIL
of the Geoprofessional Business Association

8811 Colesville Road/Suite G106, Silver Spring, MD 20910
Telephone: 301/565-2733 Facsimile: 301/589-2017
e-mail: info@geoprofessional.org www.geoprofessional.org

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CONSTRAINTS & RESTRICTIONS

The intent of this document is to bring to your attention the potential concerns and the basic limitations of a typical geotechnical report.

WARRANTY

Universal Engineering Sciences has prepared this report for our client for his exclusive use, in accordance with generally accepted soil and foundation engineering practices, and makes no other warranty either expressed or implied as to the professional advice provided in the report.

UNANTICIPATED SOIL CONDITIONS

The analysis and recommendations submitted in this report are based upon the data obtained from soil borings performed at the locations indicated on the Boring Location Plan. This report does not reflect any variations which may occur between these borings.

The nature and extent of variations between borings may not become known until excavation begins. If variations appear, we may have to re-evaluate our recommendations after performing on-site observations and noting the characteristics of any variations.

CHANGED CONDITIONS

We recommend that the specifications for the project require that the contractor immediately notify Universal Engineering Sciences, as well as the owner, when subsurface conditions are encountered that are different from those present in this report.

No claim by the contractor for any conditions differing from those anticipated in the plans, specifications, and those found in this report, should be allowed unless the contractor notifies the owner and Universal Engineering Sciences of such changed conditions. Further, we recommend that all foundation work and site improvements be observed by a representative of Universal Engineering Sciences to monitor field conditions and changes, to verify design assumptions and to evaluate and recommend any appropriate modifications to this report.

MISINTERPRETATION OF SOIL ENGINEERING REPORT

Universal Engineering Sciences is responsible for the conclusions and opinions contained within this report based upon the data relating only to the specific project and location discussed herein. If the conclusions or recommendations based upon the data presented are made by others, those conclusions or recommendations are not the responsibility of Universal Engineering Sciences.

CHANGED STRUCTURE OR LOCATION

This report was prepared in order to aid in the evaluation of this project and to assist the architect or engineer in the design of this project. If any changes in the design or location of the structure as outlined in this report are planned, or if any structures are included or added that are not discussed in the report, the conclusions and recommendations contained in this report shall not be considered valid unless the changes are reviewed and the conclusions modified or approved by Universal Engineering Sciences.

USE OF REPORT BY BIDDERS

Bidders who are examining the report prior to submission of a bid are cautioned that this report was prepared as an aid to the designers of the project and it may affect actual construction operations.

Bidders are urged to make their own soil borings, test pits, test caissons or other investigations to determine those conditions that may affect construction operations. Universal Engineering Sciences cannot be responsible for any interpretations made from this report or the attached boring logs with regard to their adequacy in reflecting subsurface conditions which will affect construction operations.

STRATA CHANGES

Strata changes are indicated by a definite line on the boring logs which accompany this report. However, the actual change in the ground may be more gradual. Where changes occur between soil samples, the location of the change must necessarily be estimated using all available information and may not be shown at the exact depth.

OBSERVATIONS DURING DRILLING

Attempts are made to detect and/or identify occurrences during drilling and sampling, such as: water level, boulders, zones of lost circulation, relative ease or resistance to drilling progress, unusual sample recovery, variation of driving resistance, obstructions, etc.; however, lack of mention does not preclude their presence.

WATER LEVELS

Water level readings have been made in the drill holes during drilling and they indicate normally occurring conditions. Water levels may not have been stabilized at the last reading. This data has been reviewed and interpretations made in this report. However, it must be noted that fluctuations in the level of the groundwater may occur due to variations in rainfall, temperature, tides, and other factors not evident at the time measurements were made and reported. Since the probability of such variations is anticipated, design drawings and specifications should accommodate such possibilities and construction planning should be based upon such assumptions of variations.

LOCATION OF BURIED OBJECTS

All users of this report are cautioned that there was no requirement for Universal Engineering Sciences to attempt to locate any man-made buried objects during the course of this exploration and that no attempt was made by Universal Engineering Sciences to locate any such buried objects. Universal Engineering Sciences cannot be responsible for any buried man-made objects which are subsequently encountered during construction that are not discussed within the text of this report.

TIME

This report reflects the soil conditions at the time of exploration. If the report is not used in a reasonable amount of time, significant changes to the site may occur and additional reviews may be required.

UES

APPENDIX D

UES™

GEOPHYSICAL INVESTIGATION

PACE ROAD INDUSTRIAL DEVELOPMENT
810 PACE ROAD
AUBURNDALE, POLK COUNTY, FLORIDA

UES PROJECT NO. A26139.00190.001
UES REPORT NO.

PREPARED FOR:

Pace Road DC Development, LLC c/o Hillwood, A Perot Company
5050 W. Tilghman
Allentown, PA 18104

Attention: Mr. Mac Utsey, Vice President

PREPARED BY:

UES Professional Solutions, LLC
3532 Maggie Boulevard
Orlando, Florida 32811
(407) 423-0504

March 23th, 2026

Materials Testing
Geotechnical Engineering
Environmental
Building Sciences & Safety
Inspections & Code Compliance
Virtual Design Consulting

March 23, 2026

Pace Road DC Development, LLC c/o Hillwood, A Perot Company
5050 W. Tilghman
Allentown, PA 18104
P: 843-276-6053

Attention: Mr. Mac Utsey, Vice President
Mac.utsey@hillwood.com

Reference: **Geophysical Investigation**
Pace Road Industrial Development
810 Pace Road
Auburndale, Polk County, Florida
UES Report No.
UES Project No.: A26139.00190.001

Dear Mr. Utsey:

UES Professional Solutions, LLC (UES) has completed a Geophysical Investigation at the above referenced site in Auburndale, Florida. The scope of our investigation was planned in conjunction with and authorized by you. This investigation was performed in accordance with UES Proposal No. 2162200v3 dated February 13th, 2026, and with generally accepted geophysical testing practices. No other warranty, express or implied, is made.

The following report presents the results of our field investigation with a geophysical interpretation of those results with respect to the project characteristics as provided to us. We have included field testing methodology, examples of the data, as well as interpretation of the data as findings.

We appreciate the opportunity to have worked with you on this project and look forward to a continued association. Please do not hesitate to contact us if you should have any questions, or if we may further assist you as your plans proceed.

Respectfully Submitted,
UES Professional Solutions, LLC (UES)

Chris Williams, P.G.
Virtual Design Consulting (VDC) Manager

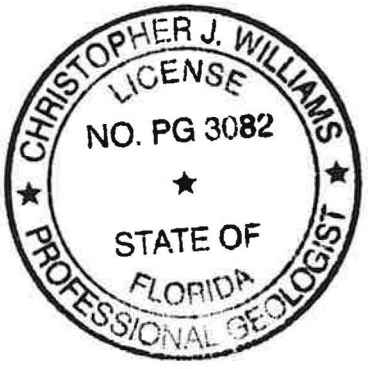


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1.0 PROJECT DESCRIPTION

UES understands that the referenced project located at 810 Pace Road, Auburndale, Polk County, Florida will require a geophysical investigation to collect soil resistivity measurements within the survey area.

Should any of the above information or assumptions made by UES be inconsistent with the planned development and construction, we request that you contact us immediately to allow us the opportunity to review the new information in conjunction with our report and revise or modify our engineering recommendations accordingly, as needed.

2.0 SITE DESCRIPTION

The survey area is located at 810 Pace Road, Auburndale, Polk County, Florida and is comprised of four separate locations highlighted by red areas in Figure 1 below.



Figure 1: Resistivity Measurement Locations

3.0 SCOPE OF SERVICES

The services conducted by UES during our non-destructive investigation were as follows:

- Collect soil resistivity measurements at four locations (RES1, RES2, RES3 and RES4) on the project site.
- Investigation will generally be in accordance with ASTM G57-20 Standard Test Method for Measurement of Soil Resistivity Using the Wenner Four-Electrode Method
- Preparation of formal report including project description, site description, scope of services, field testing, and findings.

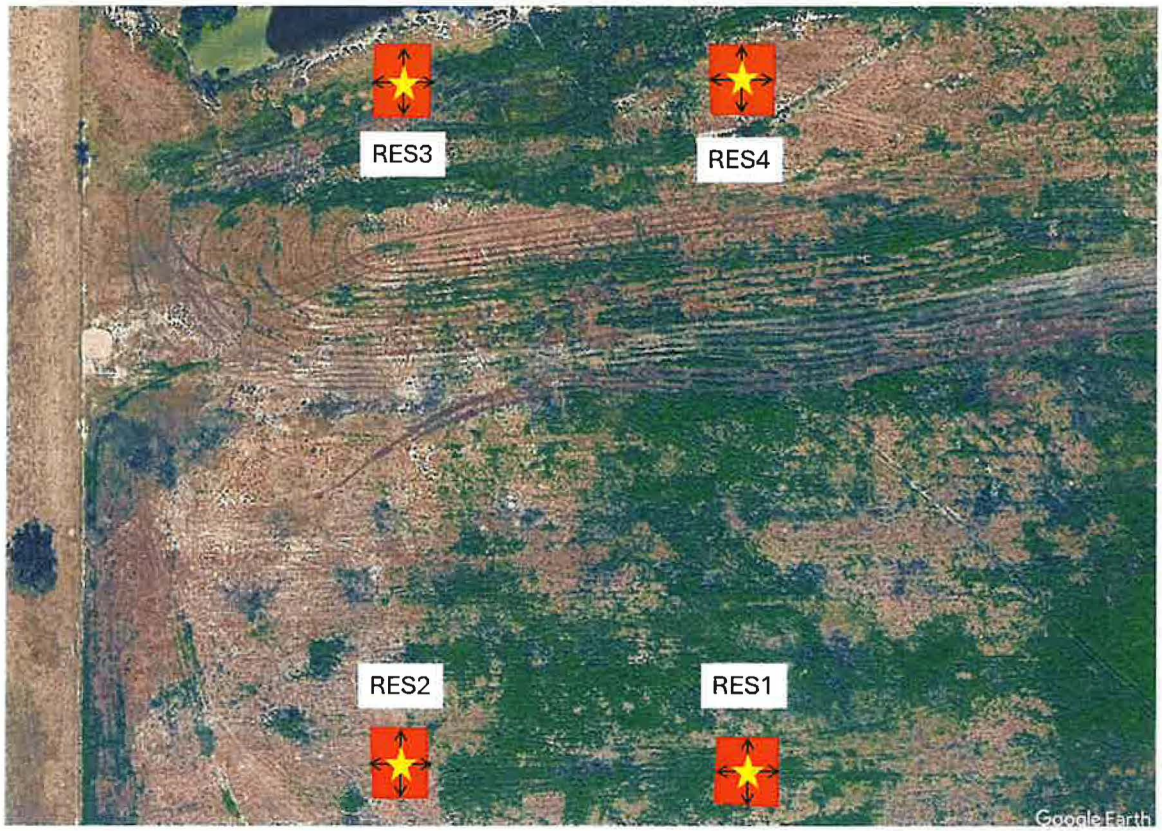
4.0 FIELD TESTING

UES visited the site on March 6th, 2026, to perform the geophysical investigation. The geophysical technique used in this investigation was Electrical Resistivity.

The measurement of soil resistivity is used for the control of corrosion of buried structures. Soil resistivity is used both for the estimation of expected corrosion rates and for the design of cathodic protection systems (ASTM G-57-20).

The Wenner four electrode method requires that four metal electrodes be placed with equal separation in a straight line in the surface of the soil to a depth not exceeding 5% of the minimum separation of the electrodes. The electrode separation should be selected with consideration of the soil strata of interest. The resulting measurement represents the average resistivity of a hemisphere of soil of a radius equal to the electrode separation. A voltage is impressed between the outer electrodes, causing current to flow, and the voltage drop between the inner electrodes is measured using a sensitive voltmeter (ASTM G-57-20).

For this investigation, electrode spacings are set as follows: 2.5', 5', 7.5', 10', 12.5', 15', 17.5' and 20'. Resistivity survey transects and example field photos are given in Figure 2, below.





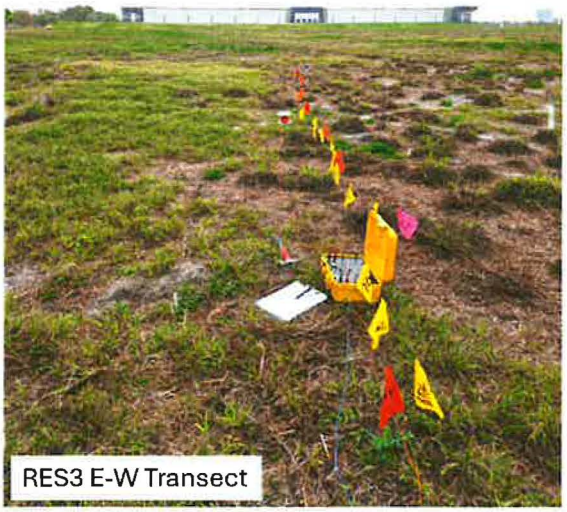


Figure 2: Resistivity Measurement Transects (top) and Example Field Photo (bottom).

5.0 FINDINGS

Measured Resistance (R) values for DSR-1 through DSR-4 in the field were used to calculate apparent resistivity (ρ) in accordance to ASTM G57-20 standard by the Equation given below:

$$\rho = 2 \times \pi \times AB \times R \text{ (ohm.ft)}$$

Apparent resistivity values are then plotted as a function of electrode spacings (AB) and given in the images below. For four test locations apparent resistivity values show an increase with larger electrode spacings (larger depths) indicating a transition from unconsolidated material (loose) to consolidated material (medium dense). These results are in agreement with the soil classification of the Boring Logs (for example boring B-34) that will be issued in UES Geotechnical report. Data spreadsheets and Apparent resistivity vs electrode spacing graphs are given in Figure 3, below.

In Situ Electrical Soil Resistivity Summary Data Form

Project Name: Pace Road Industrial Development
Project No.: A26139.00190.001
Measurment Location: RES1
Date of Measurement:March 6th, 2026

Electrode "a" spacing (ft)	Resistance (ohm)		Apparent Resistivity (ohm-ft)	
	N-S	E-W	N-S	E-W
2.5	121.60	101.70	1909.12	1596.69
5	127.40	131.60	4000.36	4132.24
7.5	154.40	140.30	7272.24	6608.13
10	152.30	144.30	9563.44	9062.04
12.5	150.70	158.60	11829.95	12450.10
15	139.90	156.20	13179.68	14714.04
17.5	123.30	159.50	13616.61	17529.05
20	122.50	158.60	15386.00	19945.28

Field Notes

- Description of Terrian: Dry soil / grass
- Difficulty of Probe Insertion: Required some force
- Weather Condition and Temperature: Mostly Cloudy / 68 °F

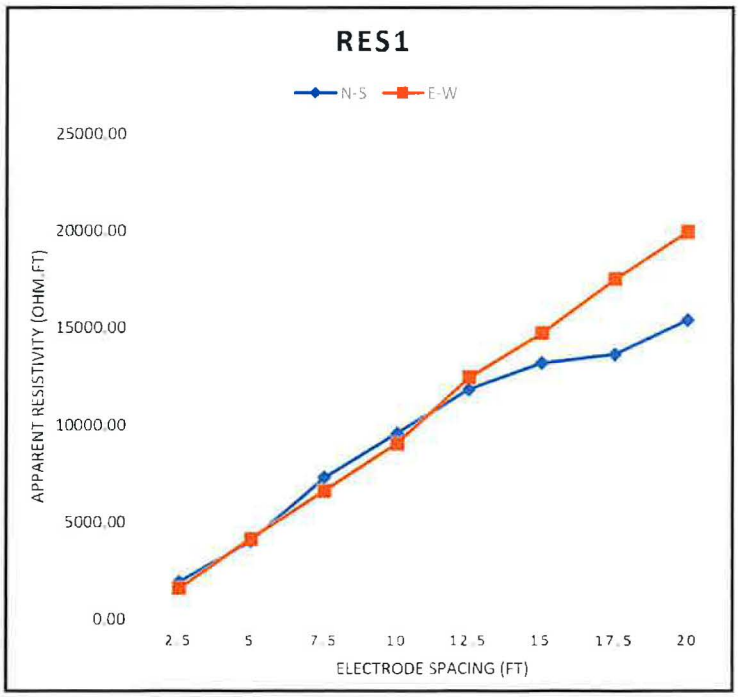


Figure 3 : Resistivity Spreadsheet and Apparent Resistivity Graph for RES1.

In Situ Electrical Soil Resistivity Summary Data Form				
Project Name: Pace Road Industrial Development				
Project No.: A26139.00190.001				
Measurment Location:	**RES2**			
Date of Measurement:March 6th, 2026				
Electrode "a" spacing (ft)	**Resistance (ohm)**		**Apparent Resistivity (ohm-ft)**	
	N-S	E-W	N-S	E-W
2.5	133.10	123.10	2089.67	1932.67
5	147.30	142.80	4625.22	4483.92
7.5	156.20	147.20	7351.02	6933.12
10	146.20	153.10	9189.36	9614.68
12.5	130.40	158.60	10236.40	12450.10
15	129.30	146.20	12180.05	13770.03
17.5	127.90	139.50	14058.21	15334.05
20	118.60	128.80	14896.16	16177.28

Field Notes

- Description of Terrian: Dry soil / grass
- Difficulty of Probe Insertion: Required some force
- Weather Condition and Temperature: Mostly Cloudy / 68 °F

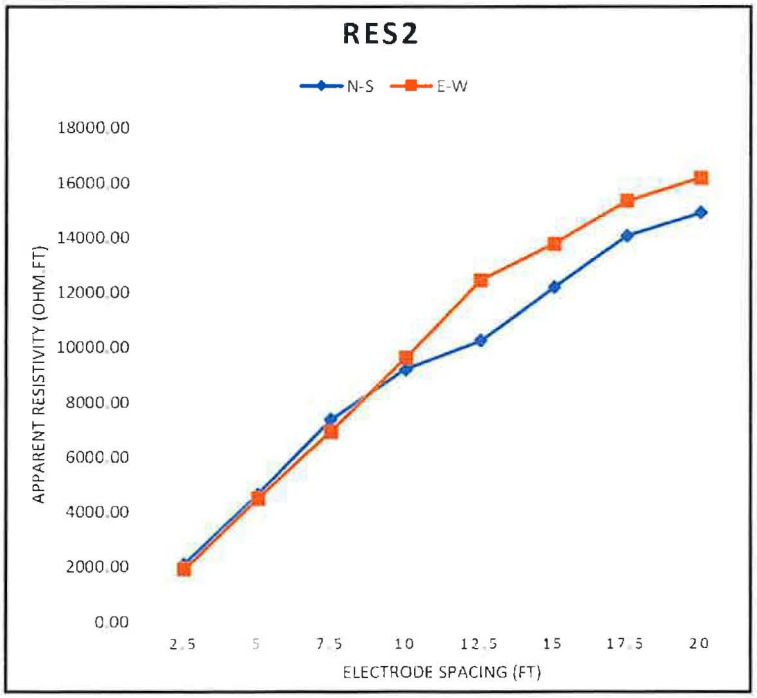


Figure 3 (cont.) : Resistivity Spreadsheet and Apparent Resistivity Graph for RES2.

In Situ Electrical Soil Resistivity Summary Data Form				
Project Name: Pace Road Industrial Development				
Project No.: A26139.00190.001				
Measurment Location:	**RES3**			
Date of Measurement:March 6th, 2026				
Electrode "a" spacing (ft)	**Resistance (ohm)**		**Apparent Resistivity (ohm-ft)**	
	N-S	E-W	N-S	E-W
2.5	137.70	132.10	2161.89	2073.97
5	147.50	138.00	4631.50	4364.60
7.5	150.30	152.40	7079.13	7178.04
10	153.60	127.70	9646.08	8019.56
12.5	152.80	131.20	11991.80	10299.20
15	154.20	134.40	14525.64	12080.48
17.5	153.60	133.30	16890.64	14649.67
20	150.50	124.80	18902.50	15674.88

Field Notes

- Description of Terrian: Dry soil / grass
- Difficulty of Probe Insertion: Required some force
- Weather Condition and Temperature: Mostly Cloudy / 68 °F

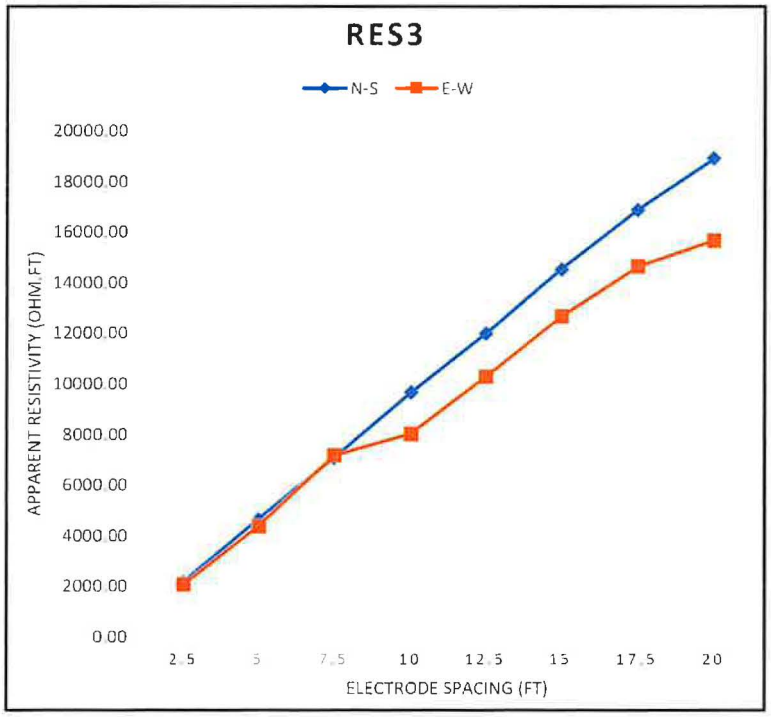


Figure 3 (cont.) : Resistivity Spreadsheet and Apparent Resistivity Graph for RES3.

In Situ Electrical Soil Resistivity Summary Data Form

Project Name: Pace Road Industrial Development				
Project No.: A26139.00190.001				
Measurment Location:	**RES4**			
Date of Measurement:March 6th, 2026				
Electrode "a" spacing (ft)	**Resistance (ohm)**		**Apparent Resistivity (ohm-ft)**	
	N-S	E-W	N-S	E-W
2.5	132.10	120.30	2073.97	1888.71
5	142.20	125.20	4466.06	3931.28
7.5	144.80	130.10	6820.08	6127.71
10	149.70	139.30	9401.15	8748.34
12.5	153.90	152.10	12051.13	11939.85
15	131.10	150.30	12349.62	14158.26
17.5	128.20	147.10	14089.18	16166.29
20	125.70	146.80	15787.92	18438.09

Field Notes

- Description of Terrian: Dry soil / grass
- Difficulty of Probe Insertion: Required some force
- Weather Condition and Temperature: Mostly Cloudy / 68 °F

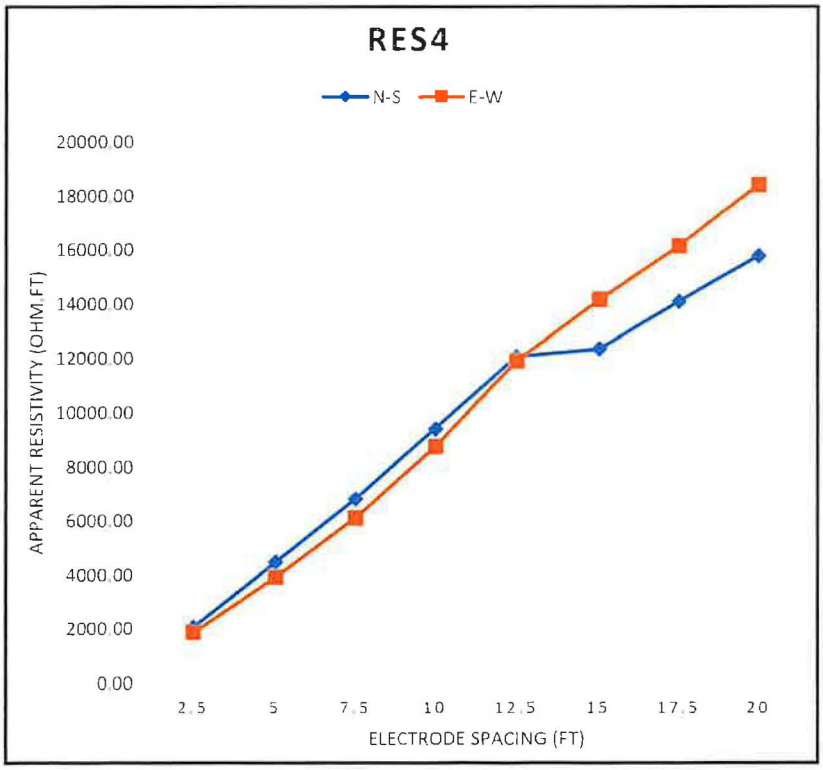


Figure 3 (cont.) : Resistivity Spreadsheet and Apparent Resistivity Graph for RES4.